H267	B331	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H268	B332	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H269	B333	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H270	B334	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H271	B335	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H272	B336	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H273	B337	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H274	B338	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H275	B339	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H276	B340	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H277	B341	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H278	B342	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H279	B343	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H280	B345	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H281	B347	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H282	B349	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H283	B351	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H284	B353	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H285	B355	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H286	B357	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H287	B359	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H288	B361	End	Rigid	Rigid	Rigid	Rigid	Free	Free
H290	B365	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H291	B366	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H292	B382	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H293	B381	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H294	B369	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H295	B370	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H296	B371	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H297	B372	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H298	B373	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H299	B374	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H301	B383	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H302	B385	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H303	B387	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H304	B389	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H305	B391	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H306	B393	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H307	B395	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H308	B399	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H309	B401	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H310	B403	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H311	B407	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H312	B408	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H313	B409	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H314	B410	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H315	B411	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H316	B412	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H317	B413	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H318	B414	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H319	B415	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H320	B417	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H321	B418	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H322	B49	Both	Rigid	Rigid	Rigid	Rigid	Free	Free
H323	B441	End	Rigid	Rigid	Rigid	Rigid	Free	Free

17.Line supports on beam

Name	Member	Pos x_1	Coor	X	Y	Z	Rx	Ry	Rz
	System	Pos x_2	Orig						
Slb2	B31	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb3	B32	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb4	B33	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free

	LCS	1.000	From start						
Slb5	B34	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb6	B35	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb7	B36	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb8	B37	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb9	B38	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb10	B39	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb11	B40	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb14	B43	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb15	B44	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb16	B45	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb17	B46	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb18	B47	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb19	B48	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb20	B367	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb21	B368	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb1	B444	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb2	B446	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						
Slb3	B445	0.000	Rela	Free	Flexible	Flexible	Free	Free	Free
	LCS	1.000	From start						

18.Surface supports on surface

Name	2D member	Type	Subsoil
SS1	S47	Individual	Sub1

1.2. ANALIZA VPLIVOV

1.3. Lastna teža konstrukcije

Lastna teža konstrukcije je upoštevana samodejno v računalniškem programu, glede na materialne in geometrijske karakteristike posameznih elementov.

1.4. Stalni vplivi

- pritličje, etaža ... g=2.00 kN /m^2
- podstrešje ... g=1.00 kN /m^2

1.5. Koristni vplivi

- skladišče v pritličju ... q=8.00 kN /m^2
- delavnica v pritličju ... q=3.00 kN /m^2
- trgovina, muzej v pritličju... q=4.00 kN /m^2
- koristna obtežba v etažah ... q=2.00 kN /m^2
- koristna obtežba v neizkoriščenem delu podstrehe ... q=1.50 kN /m^2

Obtežba predelnih sten se upošteva dodatno v velikosti 1.50 kN /m^2

1.6. Vplivi snega

Cona A2, n.m. 370m, $S = \mu S_k = 0.8 \times 1.63$ kN/m^2 = 1.30 kN/m^2

1.7. Vplivi vetra

Cona 1, hitrost vetra 20m/s, III. Kategorija terena

Vplivi vetra niso merodajni za analizo konstrukcije primarne konstrukcije, potresni vplivi so bistveno večji.

1.8. Potresni vplivi

ag=0.10g, q=1.5, tip tal B, pomembnost objekta 1.0

2. ANALIZA KONSTRUKCIJE

Solution of Free Vibration

Number of 2D elements Number of 1D elements Number of mesh nodes Number of equations	7612 2801 9703 58218
Combination of mass groups	MC 1
Number of frequencies	100
Bending theory	Mindlin
Start of calculation	21.02.2017 13:36
End of calculation	21.02.2017 13:38

Sum of masses

[kg]	X	Y	Z
Combination of mass groups 1	1448101.52	1448101.52	1448101.52

Modal participation factors

Number	Omega	Period	Freq. [Hz]	Damp ratio	Wxi / Wxtot	Wyi / Wytot	Wzi / Wztot
1	1.3413	4.6846	0.2135	0.0000	0.0009	0.0000	0.0000
2	1.3413	4.6843	0.2135	0.0000	0.0000	0.0000	0.0000
3	1.4144	4.4422	0.2251	0.0000	0.0007	0.0000	0.0000
4	1.4146	4.4418	0.2251	0.0000	0.0002	0.0000	0.0000
5	1.8086	3.4740	0.2879	0.0000	0.0003	0.0000	0.0000
6	12.3879	0.5072	1.9716	0.0000	0.0000	0.0066	0.0000
7	12.4310	0.5054	1.9785	0.0000	0.0000	0.0000	0.0000
8	12.4314	0.5054	1.9785	0.0000	0.0000	0.0000	0.0000
9	12.4316	0.5054	1.9786	0.0000	0.0000	0.0000	0.0000
10	12.4317	0.5054	1.9786	0.0000	0.0000	0.0000	0.0000
11	12.4317	0.5054	1.9786	0.0000	0.0000	0.0000	0.0000
12	12.4896	0.5031	1.9878	0.0000	0.0000	0.0011	0.0000
13	12.5015	0.5026	1.9897	0.0000	0.0000	0.0000	0.0000
14	12.5047	0.5025	1.9902	0.0000	0.0000	0.0000	0.0000
15	12.5074	0.5024	1.9906	0.0000	0.0000	0.0000	0.0000
16	16.5296	0.3801	2.6308	0.0000	0.0047	0.0000	0.0001
17	16.5349	0.3800	2.6316	0.0000	0.0003	0.0000	0.0010
18	16.5434	0.3798	2.6330	0.0000	0.0000	0.0000	0.0000
19	16.5455	0.3798	2.6333	0.0000	0.0000	0.0000	0.0000
20	16.5461	0.3797	2.6334	0.0000	0.0000	0.0000	0.0000
21	16.5469	0.3797	2.6335	0.0000	0.0000	0.0000	0.0000
22	16.6336	0.3777	2.6473	0.0000	0.0018	0.0000	0.0000
23	16.6369	0.3777	2.6479	0.0000	0.0001	0.0000	0.0003
24	16.6450	0.3775	2.6491	0.0000	0.0004	0.0000	0.0000
25	16.6460	0.3775	2.6493	0.0000	0.0000	0.0000	0.0004
26	17.8523	0.3520	2.8413	0.0000	0.0789	0.0000	0.0000
27	18.8483	0.3334	2.9998	0.0000	0.0003	0.0345	0.0000
28	18.9371	0.3318	3.0139	0.0000	0.0000	0.0000	0.0000
29	19.0972	0.3290	3.0394	0.0000	0.0002	0.0118	0.0000
30	19.1503	0.3281	3.0479	0.0000	0.0000	0.0001	0.0000
31	20.7510	0.3028	3.3026	0.0000	0.0049	0.5062	0.0008
32	21.1063	0.2977	3.3592	0.0000	0.0588	0.0134	0.0011
33	21.2121	0.2962	3.3760	0.0000	0.0003	0.0061	0.0000
34	21.3179	0.2947	3.3928	0.0000	0.0005	0.0001	0.0000
35	21.3237	0.2947	3.3938	0.0000	0.0001	0.0000	0.0000
36	21.3242	0.2947	3.3939	0.0000	0.0001	0.0000	0.0000
37	21.3262	0.2946	3.3942	0.0000	0.0000	0.0000	0.0000
38	21.3264	0.2946	3.3942	0.0000	0.0000	0.0000	0.0000
39	21.3409	0.2944	3.3965	0.0000	0.0326	0.0069	0.0002
40	21.4290	0.2932	3.4105	0.0000	0.0003	0.0018	0.0000
41	21.4514	0.2929	3.4141	0.0000	0.0295	0.0070	0.0003
42	21.6358	0.2904	3.4435	0.0000	0.0035	0.0646	0.0002
43	21.9150	0.2867	3.4879	0.0000	0.0165	0.0039	0.0000
44	21.9199	0.2866	3.4887	0.0000	0.0001	0.0000	0.0000

45	21.9211	0.2866	3.4888	0.0000	0.0041	0.0009	0.0000
46	21.9219	0.2866	3.4890	0.0000	0.0005	0.0001	0.0000
47	21.9249	0.2866	3.4895	0.0000	0.0005	0.0001	0.0000
48	21.9258	0.2866	3.4896	0.0000	0.0001	0.0000	0.0000
49	21.9264	0.2866	3.4897	0.0000	0.0015	0.0003	0.0000
50	21.9271	0.2865	3.4898	0.0000	0.0006	0.0001	0.0000
51	21.9272	0.2865	3.4898	0.0000	0.0001	0.0000	0.0000
52	21.9272	0.2865	3.4898	0.0000	0.0005	0.0001	0.0000
53	21.9282	0.2865	3.4900	0.0000	0.0097	0.0021	0.0000
54	21.9355	0.2864	3.4911	0.0000	0.1998	0.0436	0.0000
55	22.3605	0.2810	3.5588	0.0000	0.0149	0.0014	0.0000
56	22.6653	0.2772	3.6073	0.0000	0.0000	0.0000	0.0000
57	22.6762	0.2771	3.6090	0.0000	0.0005	0.0000	0.0000
58	22.7069	0.2767	3.6139	0.0000	0.0003	0.0000	0.0002
59	22.7083	0.2767	3.6141	0.0000	0.0000	0.0000	0.0000
60	22.7105	0.2767	3.6145	0.0000	0.0000	0.0000	0.0000
61	22.7195	0.2766	3.6159	0.0000	0.0000	0.0000	0.0000
62	22.7212	0.2765	3.6162	0.0000	0.0000	0.0000	0.0001
63	22.8382	0.2751	3.6348	0.0000	0.0001	0.0000	0.0000
64	22.8390	0.2751	3.6349	0.0000	0.0006	0.0000	0.0000
65	22.8835	0.2746	3.6420	0.0000	0.0000	0.0002	0.0002
66	22.8887	0.2745	3.6429	0.0000	0.0000	0.0000	0.0000
67	22.8986	0.2744	3.6444	0.0000	0.0000	0.0000	0.0000
68	22.8992	0.2744	3.6445	0.0000	0.0000	0.0000	0.0000
69	22.9003	0.2744	3.6447	0.0000	0.0000	0.0001	0.0001
70	23.0085	0.2731	3.6619	0.0000	0.0907	0.0020	0.0000
71	23.1714	0.2712	3.6878	0.0000	0.0037	0.0024	0.0000
72	23.2699	0.2700	3.7035	0.0000	0.0043	0.0129	0.0000
73	23.3133	0.2695	3.7104	0.0000	0.0003	0.0110	0.0000
74	23.5582	0.2667	3.7494	0.0000	0.1326	0.0153	0.0000
75	23.7574	0.2645	3.7811	0.0000	0.0000	0.0000	0.0000
76	23.8000	0.2640	3.7879	0.0000	0.0378	0.0016	0.0000
77	25.1313	0.2500	3.9998	0.0000	0.0002	0.0000	0.0003
78	25.1440	0.2499	4.0018	0.0000	0.0000	0.0000	0.0002
79	25.4033	0.2473	4.0431	0.0000	0.0000	0.0000	0.0000
80	25.4146	0.2472	4.0449	0.0000	0.0000	0.0000	0.0006
81	25.4233	0.2471	4.0463	0.0000	0.0000	0.0000	0.0000
82	26.3580	0.2384	4.1950	0.0000	0.1661	0.0000	0.0000
83	28.4527	0.2208	4.5284	0.0000	0.0010	0.0354	0.0007
84	28.9010	0.2174	4.5997	0.0000	0.0002	0.0006	0.0001
85	30.1411	0.2085	4.7971	0.0000	0.0003	0.0057	0.0005
86	30.2368	0.2078	4.8123	0.0000	0.0001	0.0005	0.0000
87	30.2604	0.2076	4.8161	0.0000	0.0000	0.0005	0.0009
88	30.2932	0.2074	4.8213	0.0000	0.0006	0.0000	0.0000
89	30.3974	0.2067	4.8379	0.0000	0.0000	0.0003	0.0000
90	30.9102	0.2033	4.9195	0.0000	0.0000	0.0000	0.0003
91	30.9186	0.2032	4.9209	0.0000	0.0001	0.0000	0.0002
92	31.1806	0.2015	4.9626	0.0000	0.0000	0.0006	0.0000
93	32.0143	0.1963	5.0952	0.0000	0.0001	0.0000	0.0000
94	32.2067	0.1951	5.1259	0.0000	0.0000	0.0001	0.0000
95	32.4713	0.1935	5.1680	0.0000	0.0000	0.0002	0.0000
96	32.5603	0.1930	5.1821	0.0000	0.0000	0.0004	0.0000
97	33.0041	0.1904	5.2528	0.0000	0.0000	0.0000	0.0000
98	33.0418	0.1902	5.2588	0.0000	0.0000	0.0007	0.0000
99	33.8863	0.1854	5.3932	0.0000	0.0000	0.0000	0.0000
100	34.2164	0.1836	5.4457	0.0000	0.0000	0.0000	0.0019
					0.9081	0.8038	0.0109

Seismicity

Number of 2D elements Number of 1D elements Number of mesh nodes	7612 2801 9703
Mass in analysis	Participation mass only
Signed results	No
Loadcase	Ex
Combination of mass groups	MC 1
Bending theory	Mindlin
Start of calculation	21.02.2017 13:38

End of calculation	21.02.2017 13:39

Seismicity

Number of 2D elements Number of 1D elements Number of mesh nodes	7612 2801 9703
Mass in analysis	Participation mass only
Signed results	No
Loadcase	Ey
Combination of mass groups	MC 1
Bending theory	Mindlin
Start of calculation	21.02.2017 13:39
End of calculation	21.02.2017 13:40

Linear calculation

Number of 2D elements Number of 1D elements Number of mesh nodes Number of equations	7612 2801 9703 58218
Loadcases	LC 1 LT
	LC 2 SO
	LC 3 Q
Bending theory	Mindlin
Start of calculation	21.02.2017 13:36
End of calculation	21.02.2017 13:36

Sum of loads and reactions.

	[kN]	X	Y	Z
Loadcase 1	loads	0.0	0.0	-12260.9
	reactions in nodes	0.0	0.0	-0.0
	reactions on lines	0.8	0.8	11595.9
	contact 1D	0.0	0.0	0.0
	contact 2D	-0.8	-0.8	665.0
Loadcase 2	loads	0.0	0.0	-1491.4
	reactions in nodes	-0.0	-0.0	0.0
	reactions on lines	-0.5	-0.2	1411.9
	contact 1D	0.0	0.0	0.0
	contact 2D	0.5	0.2	79.5
Loadcase 3	loads	0.0	-0.0	-2195.0
	reactions in nodes	0.0	0.0	0.0
	reactions on lines	-0.2	-0.3	2096.4
	contact 1D	0.0	0.0	0.0
	contact 2D	0.2	0.3	98.6
Loadcase 4	loads	0.0	0.0	0.0
	reactions in nodes	0.0	0.0	0.0
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Loadcase 5	loads	0.0	0.0	0.0
	reactions in nodes	0.0	0.0	0.0
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0

Dynamic loadcase: 4

Number	Freq. [Hz]	Damp ratio	Damp coef.	Sax [m/s^2]	Say [m/s^2]	Saz [m/s^2]	G(j)	Fx [kN]	Fy [kN]	Mx [kNm]	My [kNm]
1	0.2135	0.0500	1.0000	0.1960	0.0590	0.0000	3.9751	0.2609	-0.0000	0.0002	-2.6732
2	0.2135	0.0500	1.0000	0.1960	0.0590	0.0000	0.0717	0.0001	-0.0000	0.0000	-0.0009
3	0.2251	0.0500	1.0000	0.1960	0.0590	0.0000	-3.0724	0.1928	-0.0000	0.0002	-1.9727
4	0.2251	0.0500	1.0000	0.1960	0.0590	0.0000	-1.8426	0.0694	0.0000	-0.0000	-0.7102
5	0.2879	0.0500	1.0000	0.2405	0.0723	0.0000	1.4149	0.0891	-0.0000	0.0001	-0.9106
6	1.9716	0.0500	1.0000	1.9332	0.5797	0.0000	0.4205	0.2675	6.2897	-51.2446	-1.4167

7	1.9785	0.0500	1.0000	1.9399	0.5817	0.0000	0.0131	-0.0003	0.0082	-0.0689	0.0019
8	1.9785	0.0500	1.0000	1.9400	0.5817	0.0000	-0.0008	-0.0000	0.0000	-0.0003	0.0000
9	1.9786	0.0500	1.0000	1.9400	0.5817	0.0000	-0.0008	0.0000	0.0000	-0.0002	-0.0000
10	1.9786	0.0500	1.0000	1.9400	0.5817	0.0000	0.0001	0.0000	0.0000	-0.0000	-0.0000
11	1.9786	0.0500	1.0000	1.9400	0.5817	0.0000	0.0001	-0.0000	0.0000	-0.0000	-0.0000
12	1.9878	0.0500	1.0000	1.9490	0.5844	0.0000	0.1685	0.0449	1.0331	-8.3719	-0.2373
13	1.9897	0.0500	1.0000	1.9509	0.5850	0.0000	0.0052	-0.0001	0.0015	-0.0125	0.0006
14	1.9902	0.0500	1.0000	1.9514	0.5851	0.0000	0.0359	0.0018	0.0477	-0.3857	-0.0097
15	1.9906	0.0500	1.0000	1.9518	0.5853	0.0000	-0.0074	-0.0001	0.0026	-0.0217	0.0005
16	2.6308	0.0500	1.0000	1.9610	0.5880	0.0000	-0.5918	13.2872	0.1584	-1.0610	-93.3449
17	2.6316	0.0500	1.0000	1.9610	0.5880	0.0000	-0.1652	1.0142	0.0862	-0.4804	-7.5801
18	2.6330	0.0500	1.0000	1.9610	0.5880	0.0000	0.0002	0.0000	-0.0001	0.0004	-0.0002
19	2.6333	0.0500	1.0000	1.9610	0.5880	0.0000	0.0019	0.0002	-0.0003	0.0016	-0.0018
20	2.6334	0.0500	1.0000	1.9610	0.5880	0.0000	0.0023	0.0002	0.0001	-0.0004	-0.0018
21	2.6335	0.0500	1.0000	1.9610	0.5880	0.0000	0.0159	0.0097	0.0002	-0.0014	-0.0683
22	2.6473	0.0500	1.0000	1.9610	0.5880	0.0000	-0.3596	5.0230	0.0872	-0.5225	-35.1442
23	2.6479	0.0500	1.0000	1.9610	0.5880	0.0000	0.0790	0.2516	-0.0263	0.1380	-1.8910
24	2.6491	0.0500	1.0000	1.9610	0.5880	0.0000	0.1721	1.1394	0.0674	-0.4208	-7.9240
25	2.6493	0.0500	1.0000	1.9610	0.5880	0.0000	0.0188	0.0102	0.0119	-0.0639	-0.0911
26	2.8413	0.0500	1.0000	1.9610	0.5880	0.0000	-2.0649	222.4181	-5.2445	24.3663	-1454.1425
27	2.9998	0.0500	1.0000	1.9610	0.5880	0.0000	-0.4886	3.7323	38.7963	-224.5733	-16.2665
28	3.0139	0.0500	1.0000	1.9610	0.5880	0.0000	0.0169	0.0184	0.0014	0.0025	-0.0906
29	3.0394	0.0500	1.0000	1.9610	0.5880	0.0000	-0.2933	1.6496	13.9634	-79.5104	-7.3434
30	3.0479	0.0500	1.0000	1.9610	0.5880	0.0000	0.0157	-0.0017	0.0619	-0.3596	0.0036
31	3.3026	0.0500	1.0000	1.9610	0.5880	0.0000	1.5512	56.0305	571.8857	-2910.3036	-257.0594
32	3.3592	0.0500	1.0000	1.9610	0.5880	0.0000	1.4681	190.7809	91.1937	-471.1158	-886.5545
33	3.3760	0.0500	1.0000	1.9610	0.5880	0.0000	0.2188	2.1764	9.2287	-46.1282	-10.0321
34	3.3928	0.0500	1.0000	1.9610	0.5880	0.0000	0.0997	1.2373	-0.6372	3.1170	-5.8173
35	3.3938	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0329	0.1404	-0.0883	0.4355	-0.6671
36	3.3939	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0473	0.2770	-0.1362	0.6650	-1.3023
37	3.3942	0.0500	1.0000	1.9610	0.5880	0.0000	0.0234	0.0645	-0.0222	0.1073	-0.3032
38	3.3942	0.0500	1.0000	1.9610	0.5880	0.0000	0.0166	0.0343	-0.0169	0.0825	-0.1611
39	3.3965	0.0500	1.0000	1.9610	0.5880	0.0000	-0.8068	79.8572	-36.7139	178.9923	-375.7936
40	3.4105	0.0500	1.0000	1.9610	0.5880	0.0000	0.0165	0.1442	-0.3838	1.9121	-0.6813
41	3.4141	0.0500	1.0000	1.9610	0.5880	0.0000	-0.7522	71.5754	-34.9281	176.6992	-345.2080
42	3.4435	0.0500	1.0000	1.9610	0.5880	0.0000	0.6834	22.8368	97.8685	-467.6015	-100.9611
43	3.4879	0.0500	1.0000	1.9610	0.5880	0.0000	0.5404	40.1668	-19.3839	92.6569	-184.9755
44	3.4887	0.0500	1.0000	1.9610	0.5880	0.0000	0.0445	0.2711	-0.1263	0.6031	-1.2523
45	3.4888	0.0500	1.0000	1.9610	0.5880	0.0000	-0.2692	9.9461	-4.7154	22.5263	-45.7742
46	3.4890	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0943	1.2148	-0.5554	2.6481	-5.5901
47	3.4895	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0950	1.2375	-0.5823	2.7809	-5.6919
48	3.4896	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0390	0.2077	-0.0959	0.4581	-0.9594
49	3.4897	0.0500	1.0000	1.9610	0.5880	0.0000	-0.1654	3.7546	-1.7726	8.4664	-17.2779
50	3.4898	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0996	1.3609	-0.6380	3.0466	-6.2586
51	3.4898	0.0500	1.0000	1.9610	0.5880	0.0000	0.0486	0.3243	-0.1518	0.7246	-1.4912
52	3.4898	0.0500	1.0000	1.9610	0.5880	0.0000	0.0985	1.3300	-0.6228	2.9739	-6.1160
53	3.4900	0.0500	1.0000	1.9610	0.5880	0.0000	0.4154	23.6841	-11.1309	53.1563	-108.9221
54	3.4911	0.0500	1.0000	1.9610	0.5880	0.0000	1.8848	487.7843	-227.9669	1088.2480	-2240.9478
55	3.5588	0.0500	1.0000	1.9610	0.5880	0.0000	0.5216	38.2663	-11.9270	55.5047	-175.5142
56	3.6073	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0040	0.0019	0.0008	-0.0039	-0.0082
57	3.6090	0.0500	1.0000	1.9610	0.5880	0.0000	-0.1005	1.4482	-0.2836	1.2987	-6.6098
58	3.6139	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0813	0.8396	0.1889	-0.7012	-4.0013
59	3.6141	0.0500	1.0000	1.9610	0.5880	0.0000	0.0025	0.0013	-0.0017	0.0098	-0.0053
60	3.6145	0.0500	1.0000	1.9610	0.5880	0.0000	0.0228	0.0641	0.0214	-0.0856	-0.3120
61	3.6159	0.0500	1.0000	1.9610	0.5880	0.0000	0.0351	0.1508	0.0543	-0.2224	-0.7300
62	3.6162	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0254	0.0822	0.0177	-0.0432	-0.3874
63	3.6348	0.0500	1.0000	1.9610	0.5880	0.0000	0.0372	0.1799	0.0391	-0.1416	-0.8546
64	3.6349	0.0500	1.0000	1.9610	0.5880	0.0000	-0.1109	1.6941	0.0386	-0.1812	-7.8404
65	3.6420	0.0500	1.0000	1.9610	0.5880	0.0000	0.0258	0.0252	0.2252	-0.9717	-0.2122
66	3.6429	0.0500	1.0000	1.9610	0.5880	0.0000	0.0201	0.0681	-0.0392	0.1795	-0.3016
67	3.6444	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0099	0.0026	0.0371	-0.1576	-0.0276
68	3.6445	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0084	0.0084	0.0054	-0.0246	-0.0407
69	3.6447	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0045	-0.0044	0.0241	-0.1033	0.0106
70	3.6619	0.0500	1.0000	1.9610	0.5880	0.0000	1.2826	246.0623	-36.6041	164.6896	-1124.6790
71	3.6878	0.0500	1.0000	1.9610	0.5880	0.0000	0.2030	7.9854	-6.4232	28.5648	-35.4972
72	3.7035	0.0500	1.0000	1.9610	0.5880	0.0000	0.4339	18.5251	32.1111	-141.7894	-100.4843
73	3.7104	0.0500	1.0000	1.9610	0.5880	0.0000	0.2117	2.3965	14.5198	-63.8903	-20.6070
74	3.7494	0.0500	1.0000	1.9610	0.5880	0.0000	-1.7063	415.0119	140.9461	-612.8682	-1848.4412
75	3.7811	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0016	0.0004	0.0000	-0.0001	-0.0018

76	3.7879	0.0500	1.0000	1.9610	0.5880	0.0000	0.8591	113.8175	23.1651	-99.3203	-502.1360
77	3.9998	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0553	0.5498	0.2384	-1.1917	-1.8374
78	4.0018	0.0500	1.0000	1.9610	0.5880	0.0000	0.0243	0.1267	-0.0217	0.1530	-0.2010
79	4.0431	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0118	0.0277	0.0062	-0.0117	-0.3089
80	4.0449	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0058	0.0147	-0.0251	0.1355	-0.1056
81	4.0463	0.0500	1.0000	1.9610	0.5880	0.0000	0.0097	0.0195	0.0019	-0.0141	-0.1320
82	4.1950	0.0500	1.0000	1.9610	0.5880	0.0000	1.3831	471.3087	-1.6376	11.0190	-1551.6297
83	4.5284	0.0500	1.0000	1.9610	0.5880	0.0000	0.2581	8.0755	47.2888	-112.5296	-28.6449
84	4.5997	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0186	0.2661	-0.4766	0.2003	-0.9631
85	4.7971	0.0500	1.0000	1.9610	0.5880	0.0000	-0.1026	1.8931	8.4734	-18.8381	-7.3304
86	4.8123	0.0500	1.0000	1.9610	0.5880	0.0000	0.0006	0.0051	-0.0165	0.0253	-0.0129
87	4.8161	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0246	0.0812	0.5925	-0.2818	-0.2501
88	4.8213	0.0500	1.0000	1.9610	0.5880	0.0000	0.0636	1.7670	-0.1082	0.6969	-5.5726
89	4.8379	0.0500	1.0000	1.9610	0.5880	0.0000	0.0141	0.0120	0.2483	-0.3312	-0.1802
90	4.9195	0.0500	1.0000	1.9610	0.5880	0.0000	0.0067	0.0204	0.0013	0.0458	-0.0731
91	4.9209	0.0500	1.0000	1.9610	0.5880	0.0000	0.0204	0.2015	-0.0283	-0.0787	-0.7123
92	4.9626	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0218	0.0403	0.6324	-1.8768	-0.1419
93	5.0952	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0248	0.3266	0.0102	-0.0130	-0.6804
94	5.1259	0.0500	1.0000	1.9610	0.5880	0.0000	0.0103	0.0261	0.1058	0.6245	-0.0784
95	5.1680	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0067	-0.0093	0.1153	-0.1006	-0.0054
96	5.1821	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0084	-0.0240	0.2149	-0.2187	0.1013
97	5.2528	0.0500	1.0000	1.9610	0.5880	0.0000	0.0070	0.0209	0.0306	-0.0326	-0.0516
98	5.2588	0.0500	1.0000	1.9610	0.5880	0.0000	0.0174	0.0046	0.5962	-0.5993	0.0079
99	5.3932	0.0500	1.0000	1.9610	0.5880	0.0000	0.0011	0.0003	0.0017	-0.0070	0.0128
100	5.4457	0.0500	1.0000	1.9610	0.5880	0.0000	-0.0030	-0.0015	0.0260	-0.0745	-0.0038
Level=	0.00							900.73	653.72	3268.95	3962.72

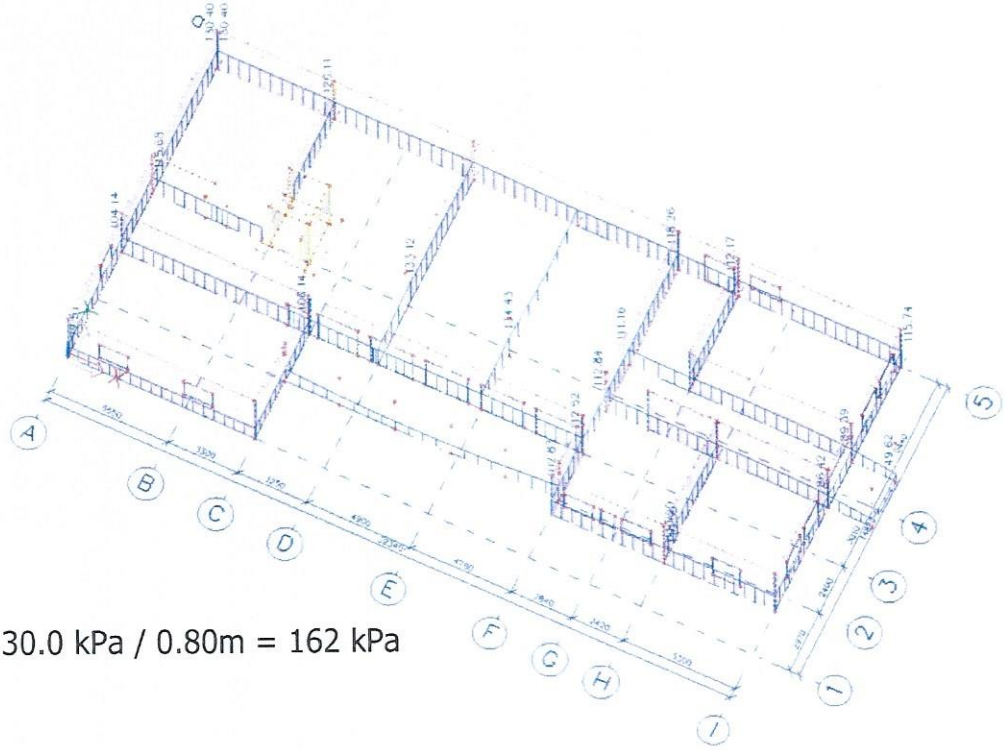
Dynamic loadcase: 5

Number	Freq. [Hz]	Damp ratio	Damp coef.	Sax [m/s^2]	Say [m/s^2]	Saz [m/s^2]	G(j)	Fx [kN]	Fy [kN]	Mx [kNm]	My [kNm]
1	0.2135	0.0500	1.0000	0.0590	0.1960	0.0000	1.1962	0.0785	-0.0000	0.0000	-0.8044
2	0.2135	0.0500	1.0000	0.0590	0.1960	0.0000	0.0215	0.0000	-0.0000	0.0000	-0.0003
3	0.2251	0.0500	1.0000	0.0590	0.1960	0.0000	-0.9242	0.0580	-0.0000	0.0001	-0.5934
4	0.2251	0.0500	1.0000	0.0590	0.1960	0.0000	-0.5547	0.0209	0.0000	-0.0000	-0.2138
5	0.2879	0.0500	1.0000	0.0723	0.2405	0.0000	0.4252	0.0268	-0.0000	0.0000	-0.2737
6	1.9716	0.0500	1.0000	0.5797	1.9332	0.0000	1.2436	0.7913	18.6033	-151.5671	-4.1903
7	1.9785	0.0500	1.0000	0.5817	1.9399	0.0000	0.0499	-0.0012	0.0310	-0.2615	0.0072
8	1.9785	0.0500	1.0000	0.5817	1.9400	0.0000	-0.0032	-0.0000	0.0001	-0.0011	0.0000
9	1.9786	0.0500	1.0000	0.5817	1.9400	0.0000	-0.0023	0.0000	0.0001	-0.0005	-0.0000
10	1.9786	0.0500	1.0000	0.5817	1.9400	0.0000	0.0002	0.0000	0.0000	-0.0000	-0.0000
11	1.9786	0.0500	1.0000	0.5817	1.9400	0.0000	0.0003	-0.0000	0.0000	-0.0000	-0.0000
12	1.9878	0.0500	1.0000	0.5844	1.9490	0.0000	0.4974	0.1324	3.0488	-24.7052	-0.7002
13	1.9897	0.0500	1.0000	0.5850	1.9509	0.0000	0.0221	-0.0004	0.0062	-0.0531	0.0025
14	1.9902	0.0500	1.0000	0.5851	1.9514	0.0000	0.1072	0.0055	0.1424	-1.1522	-0.0289
15	1.9906	0.0500	1.0000	0.5853	1.9518	0.0000	-0.0276	-0.0003	0.0097	-0.0812	0.0020
16	2.6308	0.0500	1.0000	0.5880	1.9610	0.0000	-0.1839	4.1278	0.0492	-0.3296	-28.9984
17	2.6316	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0620	0.3807	0.0324	-0.1803	-2.8449
18	2.6330	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0075	-0.0008	0.0024	-0.0129	0.0067
19	2.6333	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0027	-0.0003	0.0004	-0.0023	0.0026
20	2.6334	0.0500	1.0000	0.5880	1.9610	0.0000	0.0015	0.0001	0.0000	-0.0003	-0.0012
21	2.6335	0.0500	1.0000	0.5880	1.9610	0.0000	0.0051	0.0031	0.0001	-0.0004	-0.0219
22	2.6473	0.0500	1.0000	0.5880	1.9610	0.0000	-0.1135	1.5851	0.0275	-0.1649	-11.0905
23	2.6479	0.0500	1.0000	0.5880	1.9610	0.0000	0.0159	0.0508	-0.0053	0.0278	-0.3816
24	2.6491	0.0500	1.0000	0.5880	1.9610	0.0000	0.0607	0.4019	0.0238	-0.1484	-2.7951
25	2.6493	0.0500	1.0000	0.5880	1.9610	0.0000	0.0204	0.0111	0.0130	-0.0694	-0.0990
26	2.8413	0.0500	1.0000	0.5880	1.9610	0.0000	-0.5745	61.8845	-1.4592	6.7795	-404.5931
27	2.9998	0.0500	1.0000	0.5880	1.9610	0.0000	-1.2693	9.6956	100.7840	-583.3906	-42.2567
28	3.0139	0.0500	1.0000	0.5880	1.9610	0.0000	0.0062	0.0068	0.0005	0.0009	-0.0333
29	3.0394	0.0500	1.0000	0.5880	1.9610	0.0000	-0.7267	4.0864	34.5898	-196.9611	-18.1909
30	3.0479	0.0500	1.0000	0.5880	1.9610	0.0000	0.0570	-0.0060	0.2247	-1.3059	0.0131
31	3.3026	0.0500	1.0000	0.5880	1.9610	0.0000	4.0137	144.9807	1479.7734	-7530.5075	-665.1498
32	3.3592	0.0500	1.0000	0.5880	1.9610	0.0000	0.9988	129.7955	62.0426	-320.5181	-603.1569
33	3.3760	0.0500	1.0000	0.5880	1.9610	0.0000	0.4374	4.3502	18.4462	-92.2007	-20.0521
34	3.3928	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0254	-0.3148	0.1621	-0.7930	1.4801
35	3.3938	0.0500	1.0000	0.5880	1.9610	0.0000	0.0133	-0.0569	0.0358	-0.1764	0.2702
36	3.3939	0.0500	1.0000	0.5880	1.9610	0.0000	0.0106	-0.0623	0.0306	-0.1497	0.2931
37	3.3942	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0011	-0.0031	0.0011	-0.0052	0.0147
38	3.3942	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0038	-0.0078	0.0038	-0.0187	0.0365

39	3.3965	0.0500	1.0000	0.5880	1.9610	0.0000	0.1496	-14.8107	6.8091	-33.1967	69.6963
40	3.4105	0.0500	1.0000	0.5880	1.9610	0.0000	-0.1925	-1.6873	4.4912	-22.3741	7.9716
41	3.4141	0.0500	1.0000	0.5880	1.9610	0.0000	0.1658	-15.7746	7.6979	-38.9430	76.0809
42	3.4435	0.0500	1.0000	0.5880	1.9610	0.0000	1.3713	45.8273	196.3960	-938.3515	-202.6019
43	3.4879	0.0500	1.0000	0.5880	1.9610	0.0000	-0.1155	-8.5817	4.1414	-19.7964	39.5205
44	3.4887	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0086	-0.0523	0.0243	-0.1162	0.2413
45	3.4888	0.0500	1.0000	0.5880	1.9610	0.0000	0.0547	-2.0203	0.9578	-4.5756	9.2979
46	3.4890	0.0500	1.0000	0.5880	1.9610	0.0000	0.0172	-0.2215	0.1013	-0.4828	1.0192
47	3.4895	0.0500	1.0000	0.5880	1.9610	0.0000	0.0189	-0.2460	0.1157	-0.5527	1.1313
48	3.4896	0.0500	1.0000	0.5880	1.9610	0.0000	0.0073	-0.0390	0.0180	-0.0861	0.1803
49	3.4897	0.0500	1.0000	0.5880	1.9610	0.0000	0.0332	-0.7534	0.3557	-1.6988	3.4669
50	3.4898	0.0500	1.0000	0.5880	1.9610	0.0000	0.0196	-0.2676	0.1254	-0.5990	1.2305
51	3.4898	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0095	-0.0634	0.0297	-0.1417	0.2916
52	3.4898	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0193	-0.2606	0.1220	-0.5827	1.1985
53	3.4900	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0823	-4.6902	2.2043	-10.5266	21.5701
54	3.4911	0.0500	1.0000	0.5880	1.9610	0.0000	-0.3672	-95.0220	44.4087	-211.9943	436.5441
55	3.5588	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0068	-0.4997	0.1558	-0.7248	2.2920
56	3.6073	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0025	0.0012	0.0005	-0.0024	-0.0051
57	3.6090	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0111	0.1600	-0.0313	0.1435	-0.7304
58	3.6139	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0400	0.4128	0.0928	-0.3447	-1.9672
59	3.6141	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0038	-0.0021	0.0026	-0.0153	0.0082
60	3.6145	0.0500	1.0000	0.5880	1.9610	0.0000	0.0131	0.0369	0.0123	-0.0493	-0.1797
61	3.6159	0.0500	1.0000	0.5880	1.9610	0.0000	0.0209	0.0898	0.0324	-0.1325	-0.4348
62	3.6162	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0123	0.0397	0.0085	-0.0209	-0.1873
63	3.6348	0.0500	1.0000	0.5880	1.9610	0.0000	0.0180	0.0873	0.0190	-0.0687	-0.4148
64	3.6349	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0355	0.5429	0.0124	-0.0581	-2.5125
65	3.6420	0.0500	1.0000	0.5880	1.9610	0.0000	0.0646	0.0633	0.5652	-2.4384	-0.5324
66	3.6429	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0067	-0.0227	0.0131	-0.0598	0.1005
67	3.6444	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0273	0.0072	0.1023	-0.4342	-0.0761
68	3.6445	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0067	0.0066	0.0043	-0.0196	-0.0323
69	3.6447	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0364	-0.0363	0.1969	-0.8439	0.0865
70	3.6619	0.0500	1.0000	0.5880	1.9610	0.0000	0.2028	38.9126	-5.7886	26.0442	-177.8583
71	3.6878	0.0500	1.0000	0.5880	1.9610	0.0000	-0.1350	-5.3093	4.2706	-18.9919	23.6010
72	3.7035	0.0500	1.0000	0.5880	1.9610	0.0000	0.5805	24.7842	42.9605	-189.6960	-134.4351
73	3.7104	0.0500	1.0000	0.5880	1.9610	0.0000	0.4779	5.4100	32.7780	-144.2307	-46.5199
74	3.7494	0.0500	1.0000	0.5880	1.9610	0.0000	-0.9902	240.8586	81.8002	-355.6875	-1072.7714
75	3.7811	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0005	0.0001	0.0000	-0.0000	-0.0006
76	3.7879	0.0500	1.0000	0.5880	1.9610	0.0000	0.4076	53.9976	10.9901	-47.1199	-238.2249
77	3.9998	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0359	0.3569	0.1548	-0.7735	-1.1926
78	4.0018	0.0500	1.0000	0.5880	1.9610	0.0000	0.0033	0.0172	-0.0029	0.0208	-0.0273
79	4.0431	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0058	0.0136	0.0031	-0.0057	-0.1517
80	4.0449	0.0500	1.0000	0.5880	1.9610	0.0000	0.0167	-0.0423	0.0722	-0.3895	0.3034
81	4.0463	0.0500	1.0000	0.5880	1.9610	0.0000	0.0038	0.0075	0.0008	-0.0055	-0.0512
82	4.1950	0.0500	1.0000	0.5880	1.9610	0.0000	0.4103	139.8286	-0.4859	3.2691	-460.3398
83	4.5284	0.0500	1.0000	0.5880	1.9610	0.0000	0.5764	18.0381	105.6278	-251.3543	-63.9834
84	4.5997	0.0500	1.0000	0.5880	1.9610	0.0000	0.0599	-0.8572	1.5353	-0.6454	3.1027
85	4.7971	0.0500	1.0000	0.5880	1.9610	0.0000	-0.2093	3.8602	17.2783	-38.4132	-14.9476
86	4.8123	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0545	-0.4340	1.3975	-2.1414	1.0898
87	4.8161	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0586	0.1934	1.4122	-0.6715	-0.5961
88	4.8213	0.0500	1.0000	0.5880	1.9610	0.0000	0.0154	0.4295	-0.0263	0.1694	-1.3547
89	4.8379	0.0500	1.0000	0.5880	1.9610	0.0000	0.0411	0.0351	0.7232	-0.9646	-0.5248
90	4.9195	0.0500	1.0000	0.5880	1.9610	0.0000	0.0024	0.0073	0.0005	0.0164	-0.0261
91	4.9209	0.0500	1.0000	0.5880	1.9610	0.0000	0.0034	0.0335	-0.0047	-0.0131	-0.1186
92	4.9626	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0612	0.1129	1.7728	-5.2610	-0.3979
93	5.0952	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0081	0.1071	0.0033	-0.0043	-0.2231
94	5.1259	0.0500	1.0000	0.5880	1.9610	0.0000	0.0202	0.0513	0.2078	1.2273	-0.1540
95	5.1680	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0297	-0.0416	0.5139	-0.4485	-0.0242
96	5.1821	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0432	-0.1235	1.1047	-1.1240	0.5208
97	5.2528	0.0500	1.0000	0.5880	1.9610	0.0000	0.0086	0.0256	0.0375	-0.0400	-0.0633
98	5.2588	0.0500	1.0000	0.5880	1.9610	0.0000	0.0566	0.0149	1.9430	-1.9531	0.0259
99	5.3932	0.0500	1.0000	0.5880	1.9610	0.0000	0.0024	0.0007	0.0037	-0.0155	0.0283
100	5.4457	0.0500	1.0000	0.5880	1.9610	0.0000	-0.0122	-0.0063	0.1062	-0.3039	-0.0155
Level=	0.00							369.53	1505.85	7642.18	1641.39

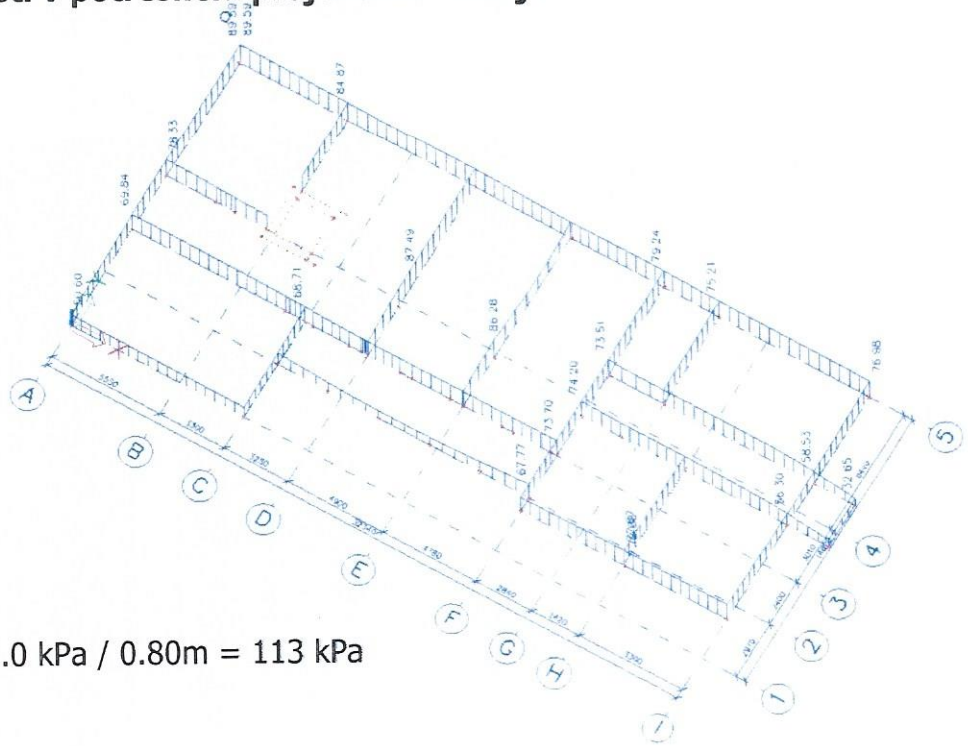
2.1. NAPETOSTI V TEMELJNIH TLEH

Napetosti v stalnem in začasnem projektnem stanju (MSN)



q_{max}=130.0 kPa / 0.80m = 162 kPa

Napetosti v potresnem projektnem stanju



q_{max}=90.0 kPa / 0.80m = 113 kPa

2.2. KONTROLA DEFORMACIJ ETAŽNE PLOŠČE

Določitev faktorja lezenja

PODATKI			
f_{ck}	25	MPa	...katakteristična tlačna trdnost betona
A_c	0.20	m^2	...površina prereza
u	2.00	m	...obseg dela prereza, ki je izpostavljen sušenju
RH	80	%	...relativna vlažnost okolice
cement	N (normal)		...vrsta cementa (S,N,R)
t_s	28	dni	...čas prve obremenitve betona
t_0	7	dni	...starost betona po končani negi
t	36500	dni	...starost betona v obravnavanem času

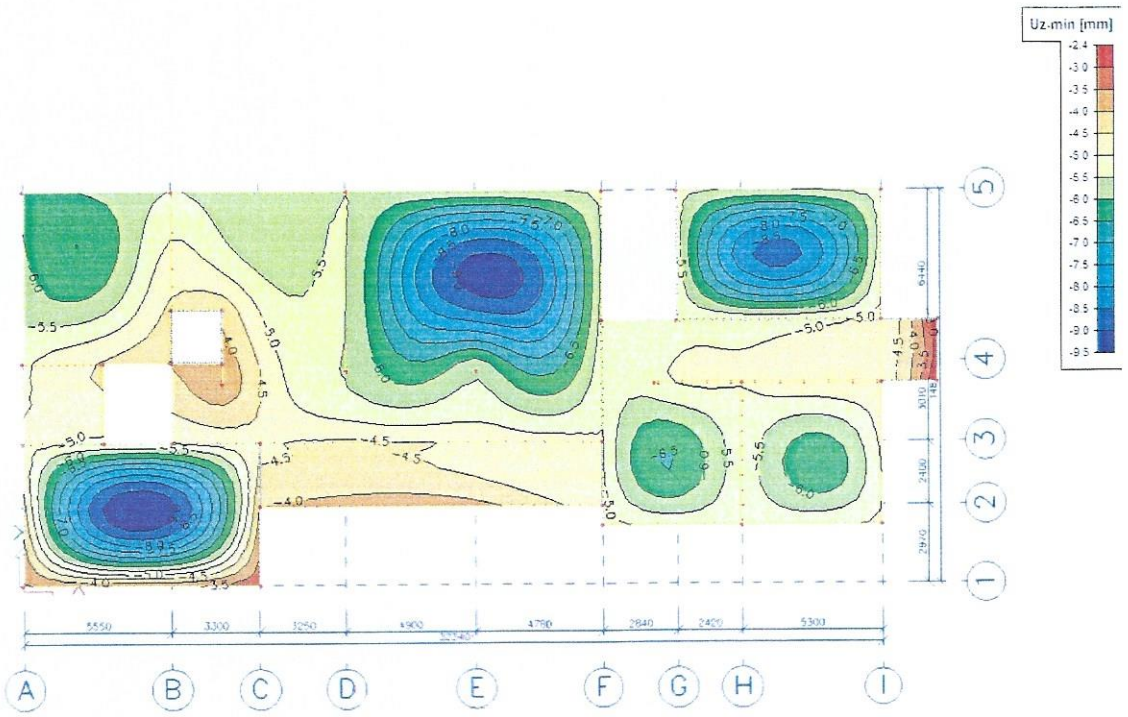
KRČENJE BETONA		
f_{cm}	33	
h_0	200.0	
k_h	0.850	
$\beta_{ds}(t,t_s)$	0.997	
$\beta_{as}(t)$	1.000	
β_{RH}	0.756	
$\varepsilon_{cd,0}$	0.000286	
$\varepsilon_{ca}(inf)$	0.000038	
$\varepsilon_{ca}(t)$	0.000038	...deformacija zaradi avtogenega krčenja
$\varepsilon_{cd}(t)$	0.000242	...deformacija krčenja zaradi sušenja
$\varepsilon_{cs}(t)$	0.000279	...celotna deformacija krčenja

LEZENJE BETONA		
α_1	1.000	
α_2	1.000	
α_3	1.000	
φ_{RH}	1.342	
$\beta(f_{cm})$	2.925	
$\beta(t_0)$	0.488	
β_H	693.9	
$\beta_c(t,t_0)$	0.994	
φ_0	1.917	
$\varphi(t,t_0)$	1.906	...koeficient lezenja betona

Redukcija modula elastičnosti na račun lezenja betona		
$\rho=1/(1+\varphi)$	0.344	
Ekvivalentna temperaturna ohladitev za zajem krčenja		
$T=T_{ekv,cs}*\rho$	-9.62	°C

ČASOVNI RAZVOJ TRDNOSTI BETONA		
$\beta_{cc}(t)$	1.000	
$f_{cm}(t)$[MPa]	33.00	...srednja tlačna trdnost pri starosti 28 dni
$f_{ck}(t)$[MPa]	25.00	...karakteristična tlačna trdnost pri starosti 7 dni

$$\phi=1+\varphi=1+1.91=2.91$$



Elastične deformacije(cm)

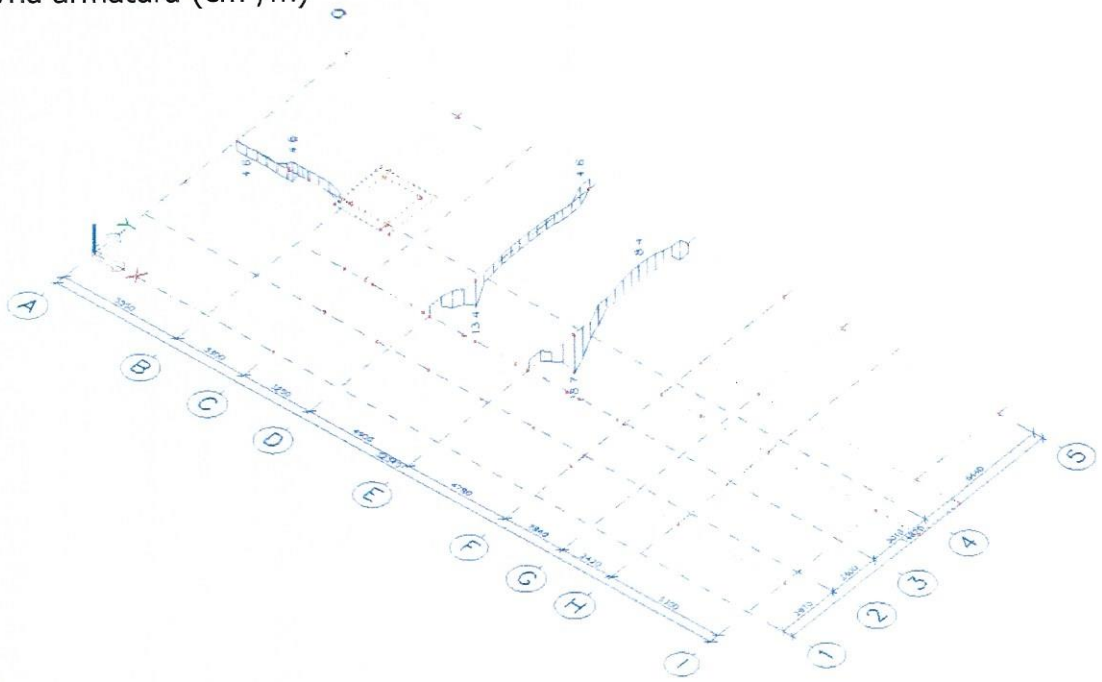
$\Delta U_{z,el}$=9.5-5.5=4 mm

$\Delta U_{z,reo}$= ϕ × $\Delta U_{z,el}$ =2.91×4=11.6 mm < L/300=9450/300=31.5 mm ... OK!

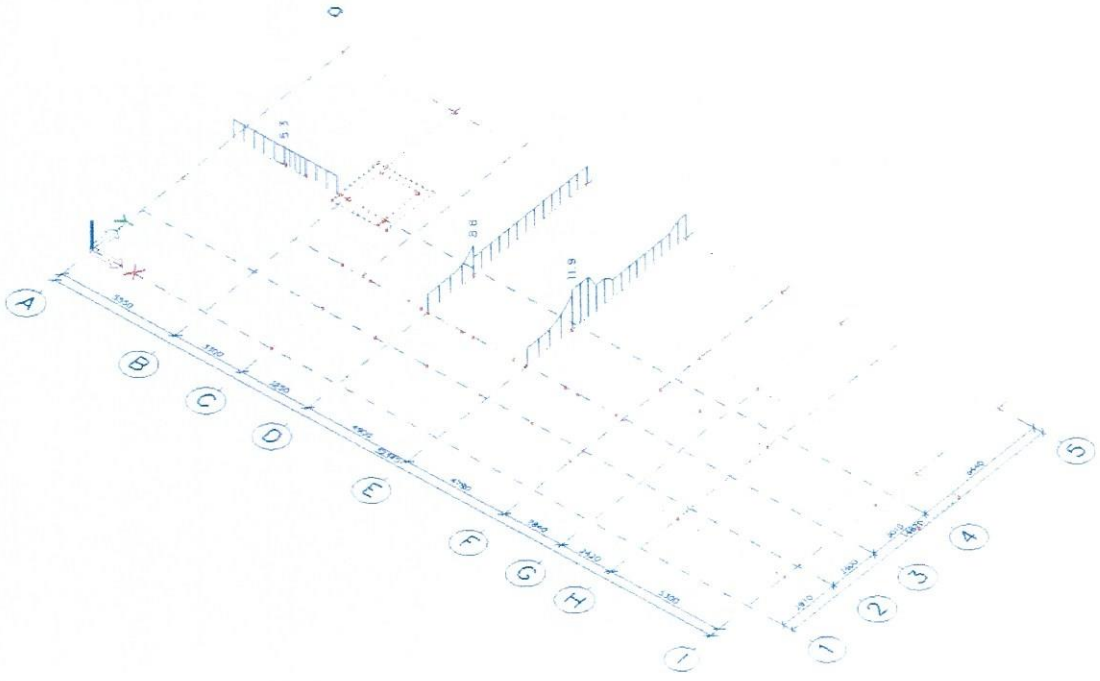
2.3. DIMENZIONIRANJE ELEMENTOV

2.3.1. Novi pasovni temelji

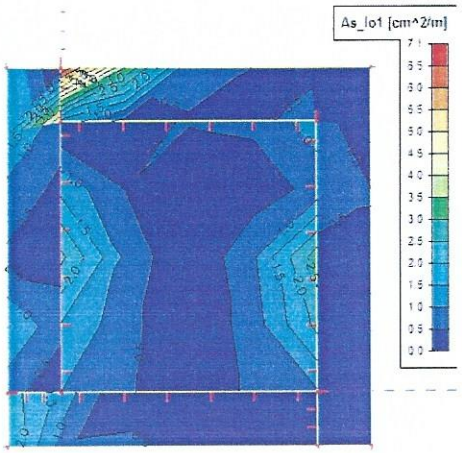
Glavna armatura (cm²/m)

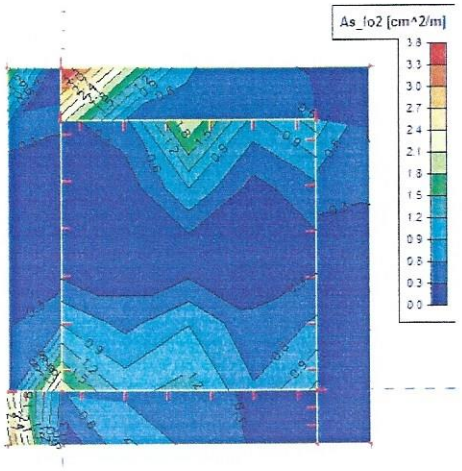


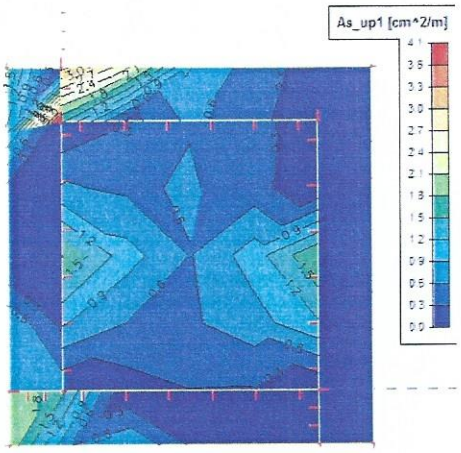
stižna armatura (cm²/m)

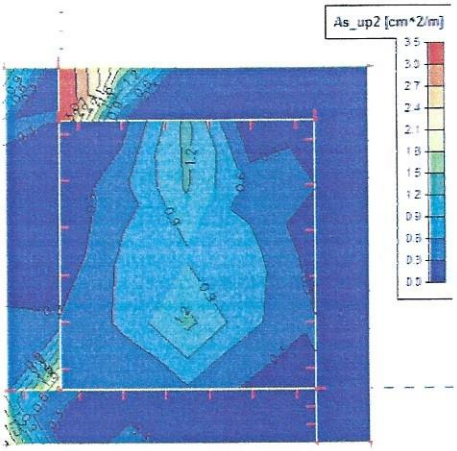


2.3.2. Plošča dvigalnega jaška

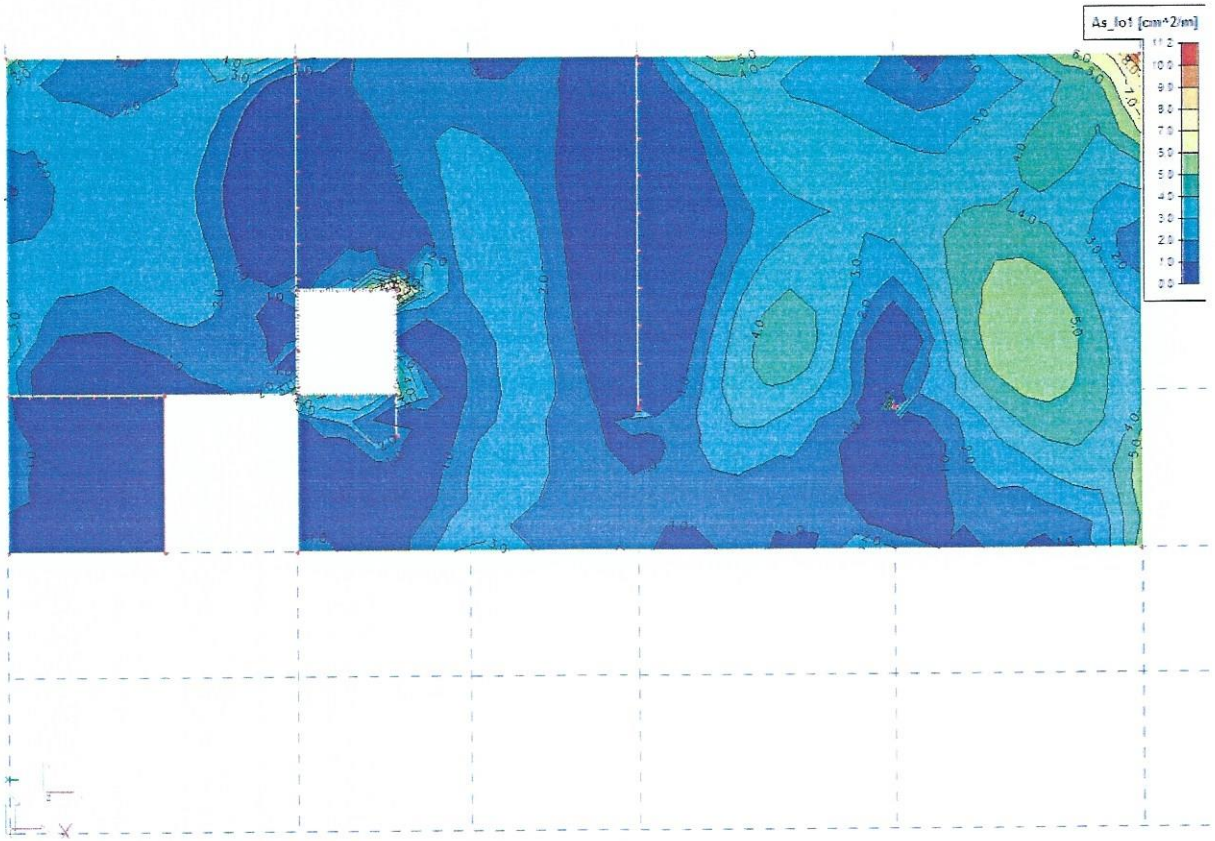


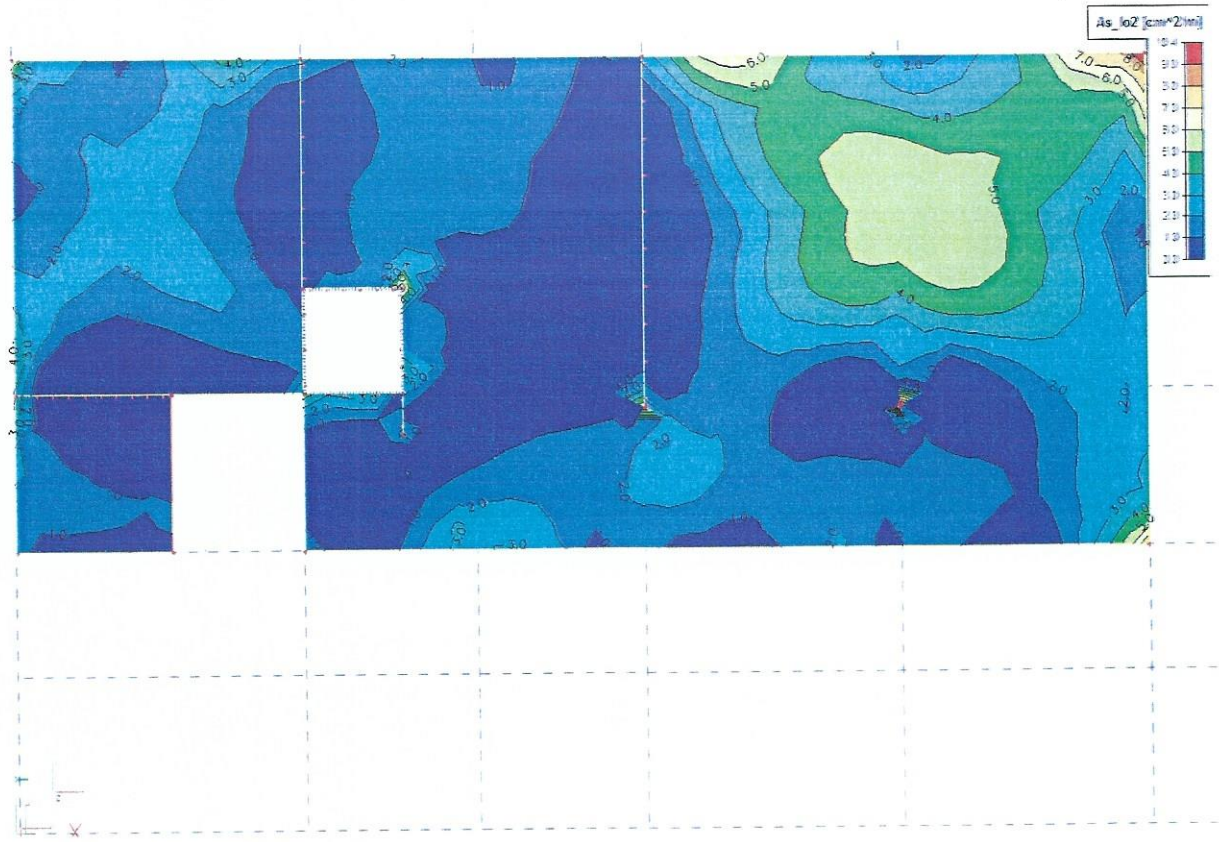


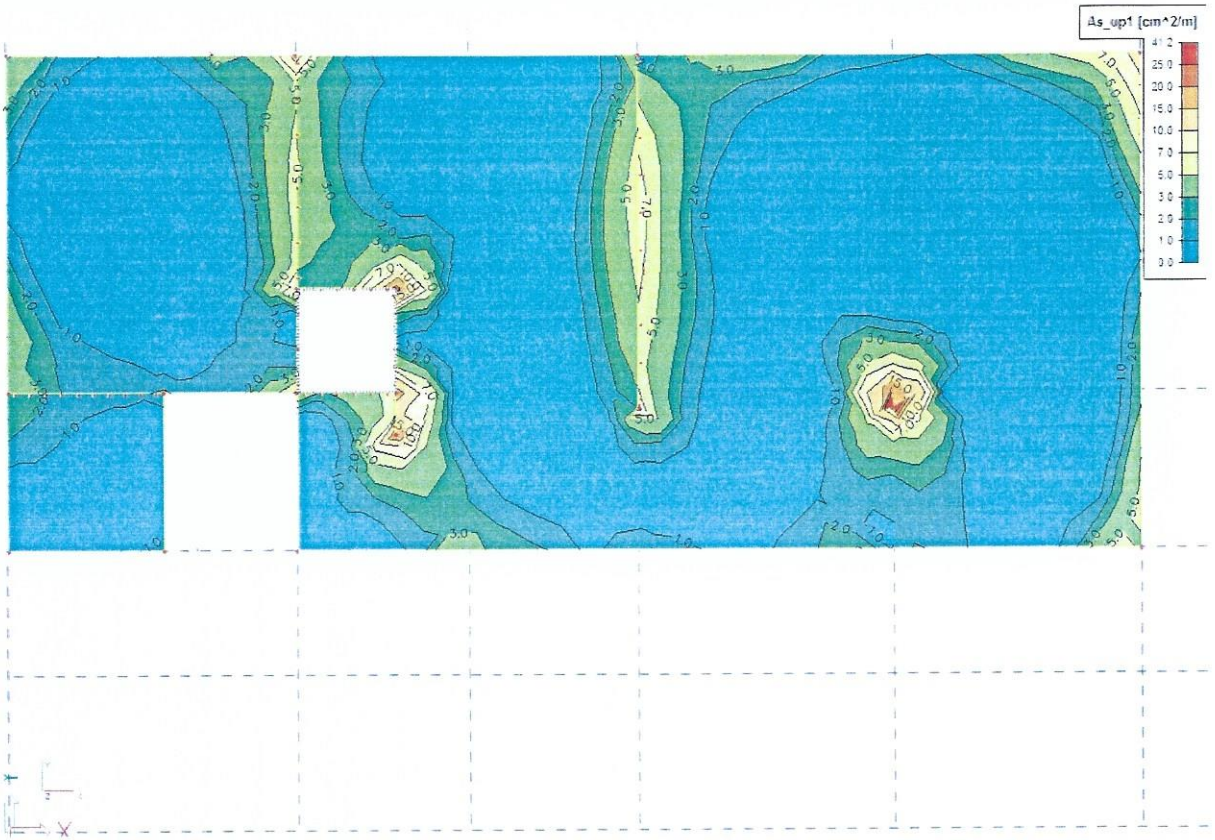


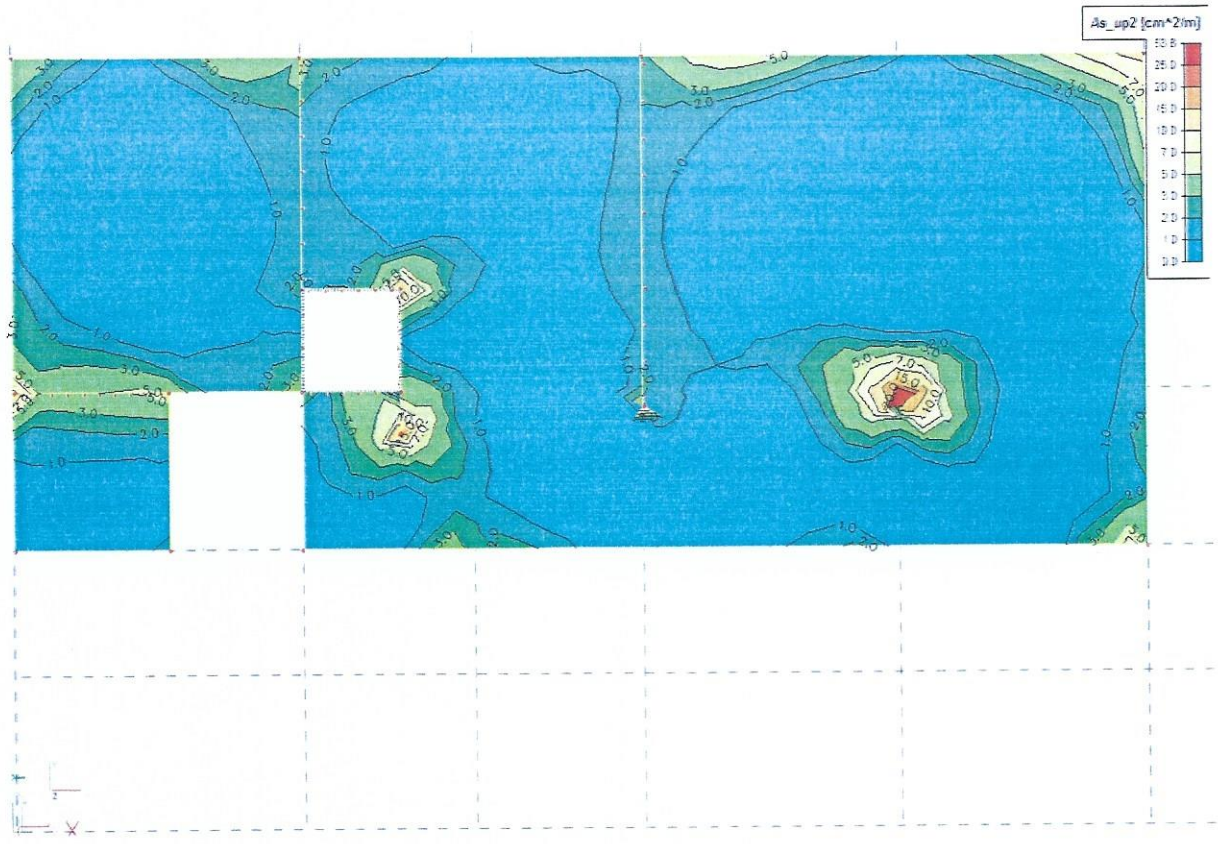


2.3.3. Nova plošča nad pritličjem

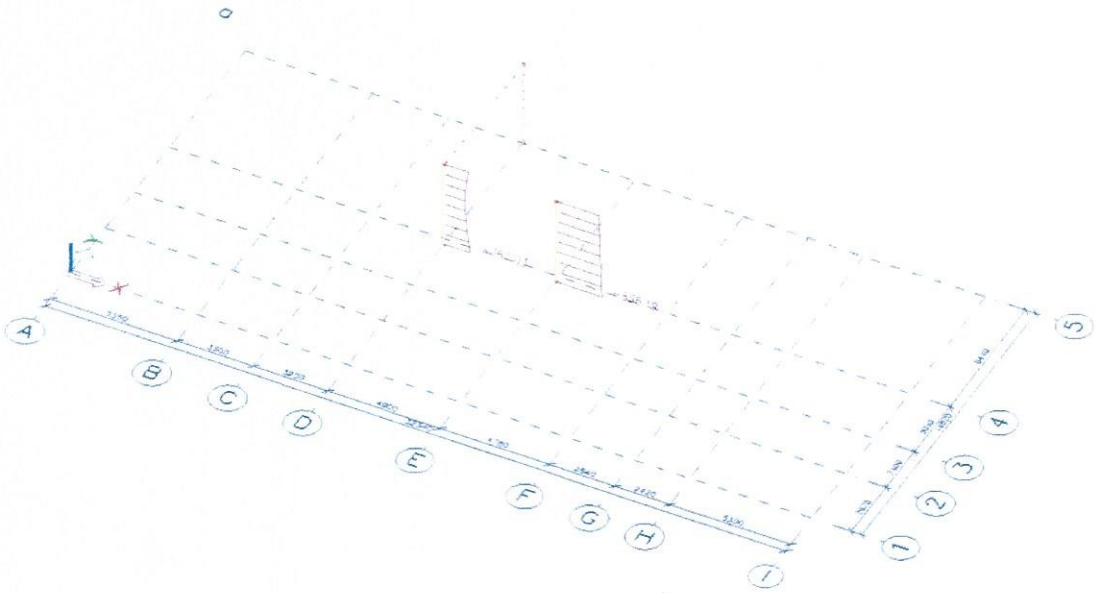





As_up1 [cm^2/m]


As_up2 [cm^2/m]

Kontrola preboja



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Loads

Punching shear load	V_{Ed} = 655 kN
Share of dynamic load	$V_{Ed,dyn}$ = 0 kN

Punching shear load referred to critical perimeter

Increase factor for design effective shear	β = 1.35

Dimensions - Internal column Rectangular column

Column width	a = 1000 mm
Column thickness	b = 300 mm
Slab thickness	h = 200 mm
Effective height	d = 170 mm
concrete cover norm co/cu	co; cu = 25; 25 mm

Material

Concrete quality	C25/30 (f_{ck} = 25.0 N/mm²)
Steel quality	B500 (f_{yk} = 500 N/mm²)
Reinforcement ratio	$\rho=(\rho_x \cdot \rho_y)^{1/2}=(1.63 \cdot 1.63)^{1/2}$ = 1.63 %

A_{sx} = 27.7 cm²/m (~ø20/113 mm); A_{sy} = 27.7 cm²/m (~ø20/113 mm)

Longitudinal reinforcement must be anchored outside the outer perimeter "Uout"

Emergency reinforcement above the column:

$V_{Ed} / 1,4 / f_{yk}$ = 9.4 cm²

Punching resistance design according to DIN EC2 + NA:2013 + ETA

Factor κ	$\kappa = \min\{1+(200/d)^{1/2}; 2\}$ = 2.00
Effect of slab thickness	η = 1+(d-200)/1000 {min 1,0; max 1,6} = 1.00
Factor $C_{Rd,c}$	$C_{Rd,c} = 0.18/\gamma_c$ = 0.12
Minimal concrete load capacity	$v_{min} = (0,0525/\gamma_c) \cdot \kappa^{3/2} \cdot fck^{1/2}$ = 495.0 kN/m²
Slab resistance against shear	$v_{Rd,c} = \max\{C_{Rd,c} \cdot \kappa \cdot (\rho \cdot f_{ck})^{1/3}; v_{min}\}$ = 825.7 kN/m²

Critical perimeter u_{crit}

Critical distance	a_{crit} =2.0d = 340 mm
Length of perimeter	u_{crit} = 3.936 m
Load within critical perimeter	$V_{Ed,\beta} = \beta \cdot V_{Ed}$ = 884.3 kN
Slab resistance against shear	$V_{Rd,c,crit} = v_{Rd,c} \cdot d \cdot U_{crit}$ = 552.6 kN
Maximum slab resistance	$V_{Rd,max,crit} = V_{Rd,c,crit,(CRdc=0.12)} \cdot 1.96$ = 1083.0 kN

$V_{Rd,c,crit}$=552.6kN ≤ $V_{Ed,\beta}$=884.3kN ≤ $V_{Rd,max,crit}$=1083.0kN

Punching shear reinforcement is required, selected:

16x Schöck BOLE 10/150-7/A770-CV25

Proof of the steel load capacity

$V_{Ed,\beta}$=884.3kN ≤ $V_{Rd,sy,crit}$=$m_c \cdot n_c \cdot A_{s,i} \cdot f_{yd}/\eta$=1093kN

Outer perimeter u_{out} (with ls + 1.5d)

Length of the reinforced area	ls = 715 mm
Length of perimeter	u_{out} = 7.895 m
Increase factor for design effective shear	$\beta_{red} = \beta$ = 1.35
Load within critical perimeter	$V_{Ed,out} = \beta_{red} \cdot V_{Ed}$ = 884.3 kN
Slab resistance against shear	$v_{Rd,c,out} = \max\{C_{Rd,c,out} \cdot \kappa \cdot (\rho \cdot f_{ck})^{1/3}; v_{min}\}$ = 688.1 kN/m²
Slab resistance against shear	$V_{Rd,c,out} = v_{Rd,c,out} \cdot d \cdot u_{out}$ = 923.5 kN

$V_{Ed,out}$=884.3kN ≤ $V_{Rd,c,out}$=923.5kN

Sufficient length of the punching reinforcement

-/-

Date: 21.2.2017

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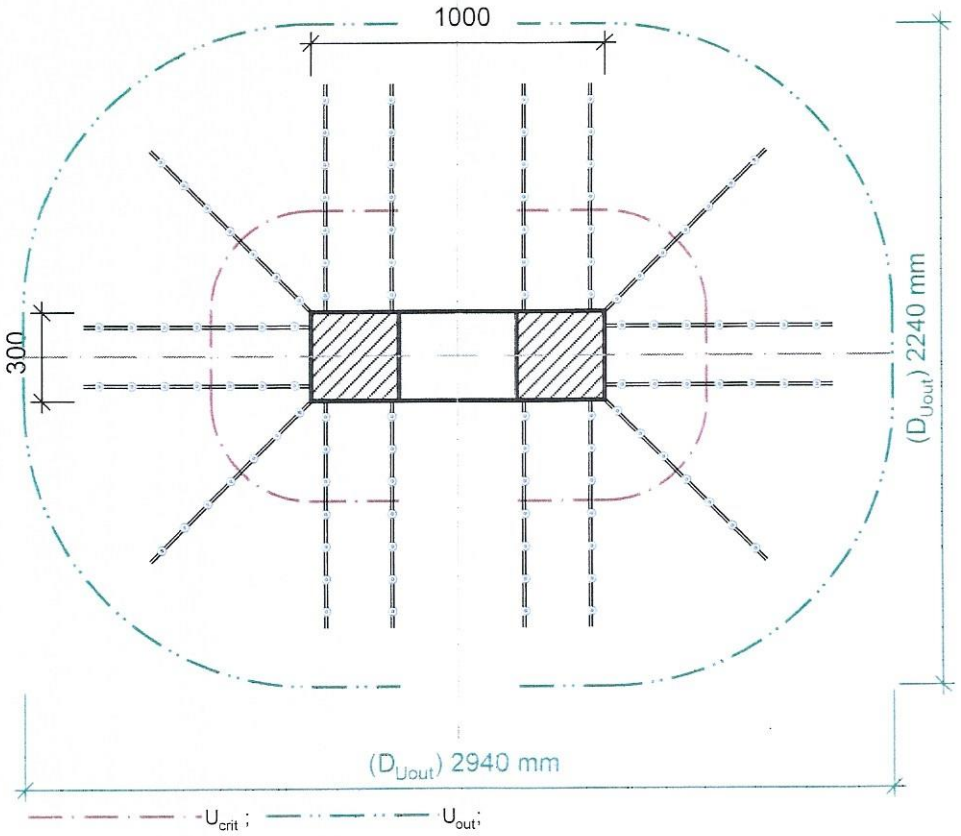
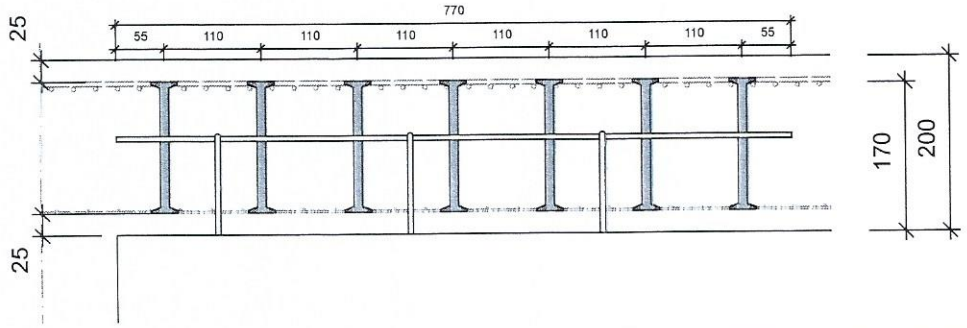


16x Schöck BOLE 10/150-7/A770-CV25



-/-

Date: 21.2.2017

Schöck BOLE Version : 2.10.04

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Loads

Punching shear load	V_{Ed} = 365 kN
Share of dynamic load	$V_{Ed,dyn}$ = 0 kN
Increase factor for design effective shear	β = 1.35

Dimensions - End of wall

Wall thickness	b = 300 mm
Slab thickness	h = 200 mm
Effective height	d = 170 mm
concrete cover nom co/cu	co; cu = 25; 25 mm

Material

Concrete quality	C25/30 (f_{ck} = 25.0 N/mm²)
Steel quality	B500 (f_{yk} = 500 N/mm²)
Reinforcement ratio	$\rho=(\rho_x \cdot \rho_y)^{1/2}=(2.30 \cdot 1.15)^{1/2}$ = 1.63 %

A_{sx} = 39.2 cm²/m (~ø20/80 mm); A_{sy} = 19.6 cm²/m (~ø20/160 mm)

Longitudinal reinforcement must be anchored outside the outer perimeter "Uout"

Emergency reinforcement above the column:

V_{Ed} / 1,4 / f_{yk} = 5.2 cm²

Punching resistance design according to DIN EC2 + NA:2013 + ETA

Factor κ	$\kappa = \min\{1+(200/d)^{1/2}; 2\}$ = 2.00
Effect of slab thickness	η = 1+(d-200)/1000 {min 1,0; max 1,6} = 1.00
Factor $C_{Rd,c}$	$C_{Rd,c} = 0.18/\gamma_c$ = 0.12
Minimal concrete load capacity	$v_{min} = (0{,}0525/\gamma_c) \cdot \kappa^{3/2} \cdot fck^{1/2}$ = 495.0 kN/m²
Slab resistance against shear	$v_{Rd,c} = \max\{C_{Rd,c} \cdot \kappa \cdot (\rho \cdot f_{ck})^{1/3}; v_{min}\}$ = 825.7 kN/m²

Critical perimeter u_{crit}

Critical distance	a_{crit} =2.0d = 340 mm
Length of perimeter	u_{crit} = 1.968 m
Load within critical perimeter	$V_{Ed,ß} = ß \cdot V_{Ed}$ = 492.8 kN
Slab resistance against shear	$V_{Rd,c,crit} = v_{Rd,c} \cdot d \cdot U_{crit}$ = 276.3 kN
Maximum slab resistance	$V_{Rd,max,crit} = V_{Rd,c,crit,(CRdc=0.12)} \cdot 1.96$ = 541.5 kN

$V_{Rd,c,crit}$=276.3kN ≤ $V_{Ed,ß}$=492.8kN ≤ $V_{Rd,max,crit}$=541.5kN

Punching shear reinforcement is required, selected:

7x Schöck BOLE 12/150-8/A880-CV25

Proof of the steel load capacity

$V_{Ed,ß}$=492.8kN ≤ $V_{Rd,sy,crit}=m_c \cdot n_c \cdot A_{s,i} \cdot f_{yd}/\eta$=688kN

Outer perimeter u_{out} (vorh ls + 1.5d)

Length of the reinforced area	ls = 825 mm
Length of perimeter	u_{out} = 4.293 m
Increase factor for design effective shear	$ß_{red}$ = ß = 1.35
Load within critical perimeter	$V_{Ed,out} = ß_{red} \cdot V_{Ed}$ = 492.8 kN
Slab resistance against shear	$v_{Rd,c,out} = \max\{C_{Rd,c,out} \cdot \kappa \cdot (\rho \cdot f_{ck})^{1/3}; v_{min}\}$ = 688.1 kN/m²
Slab resistance against shear	$V_{Rd,c,out} = v_{Rd,c,out} \cdot d \cdot u_{out}$ = 502.2 kN

$V_{Ed,out}$=492.8kN ≤ $V_{Rd,c,out}$=502.2kN

Sufficient length of the punching reinforcement

-/-

Date: 21.2.2017

Schöck BOLE Version : 2.10.04

Schöck

001 Projekt

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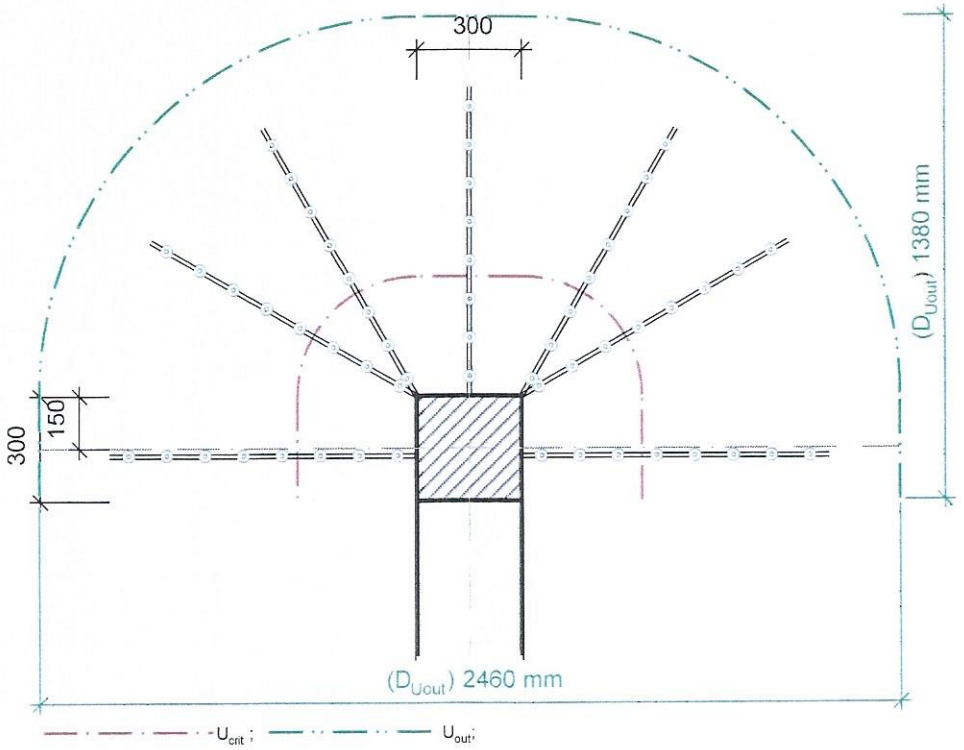
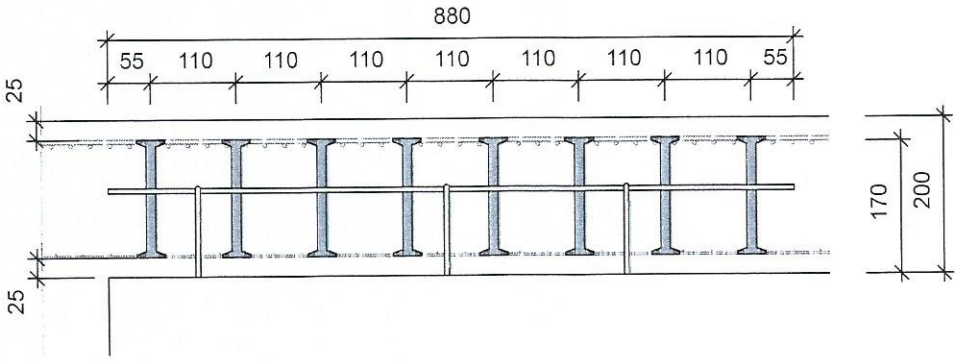
Sheet: 2



7x Schöck BOLE 12/150-8/A880-CV25

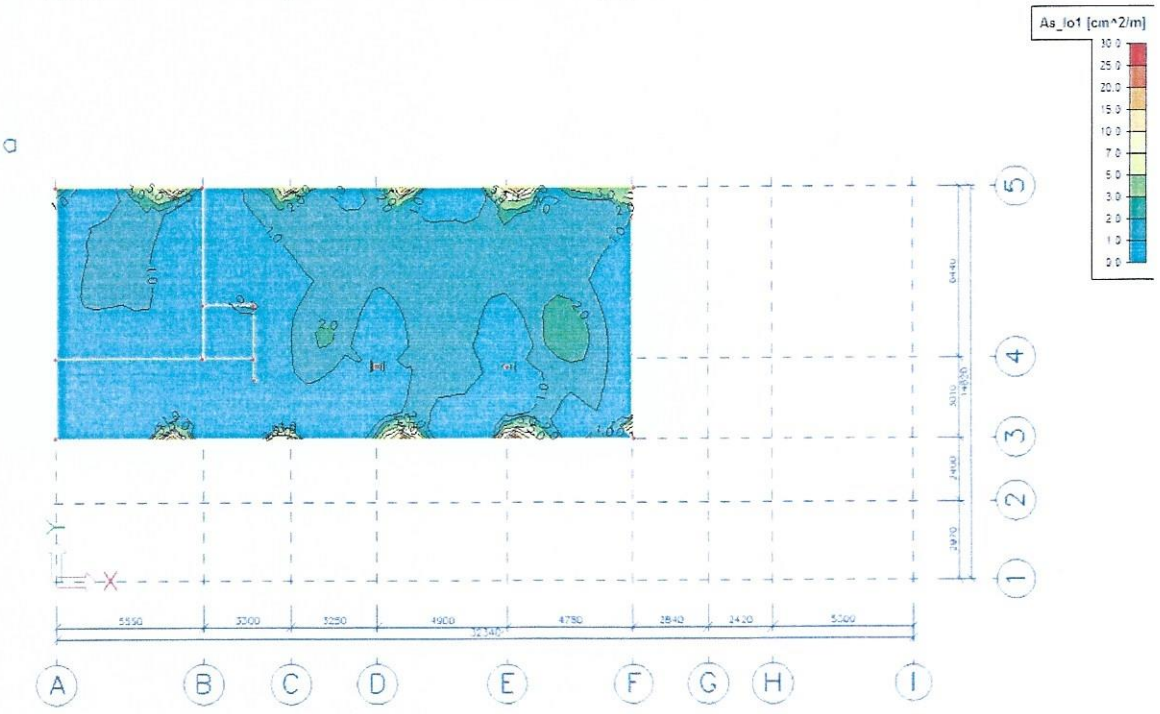


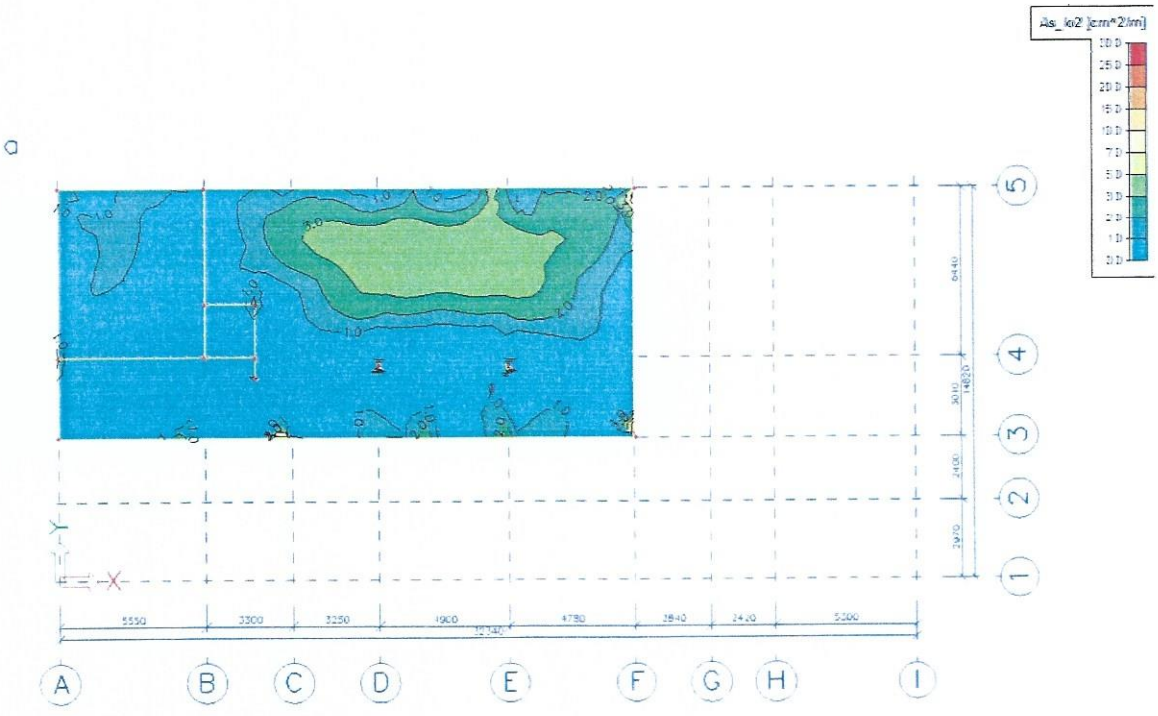
-/-

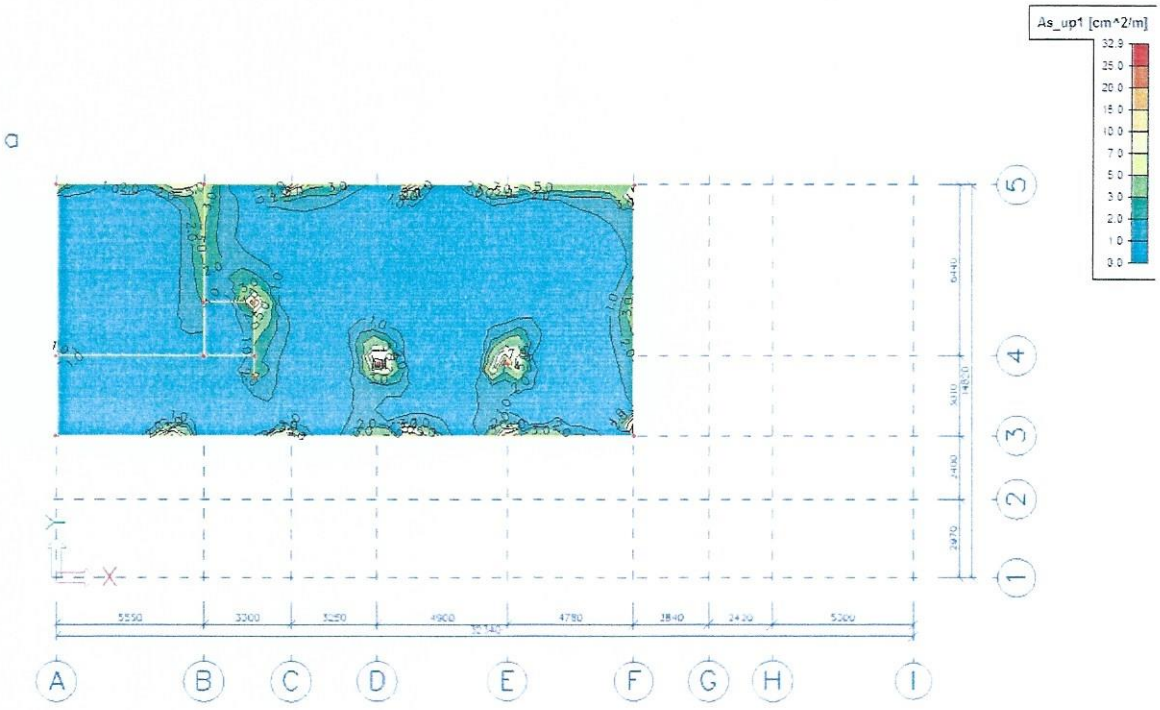
Date: 21.2.2017

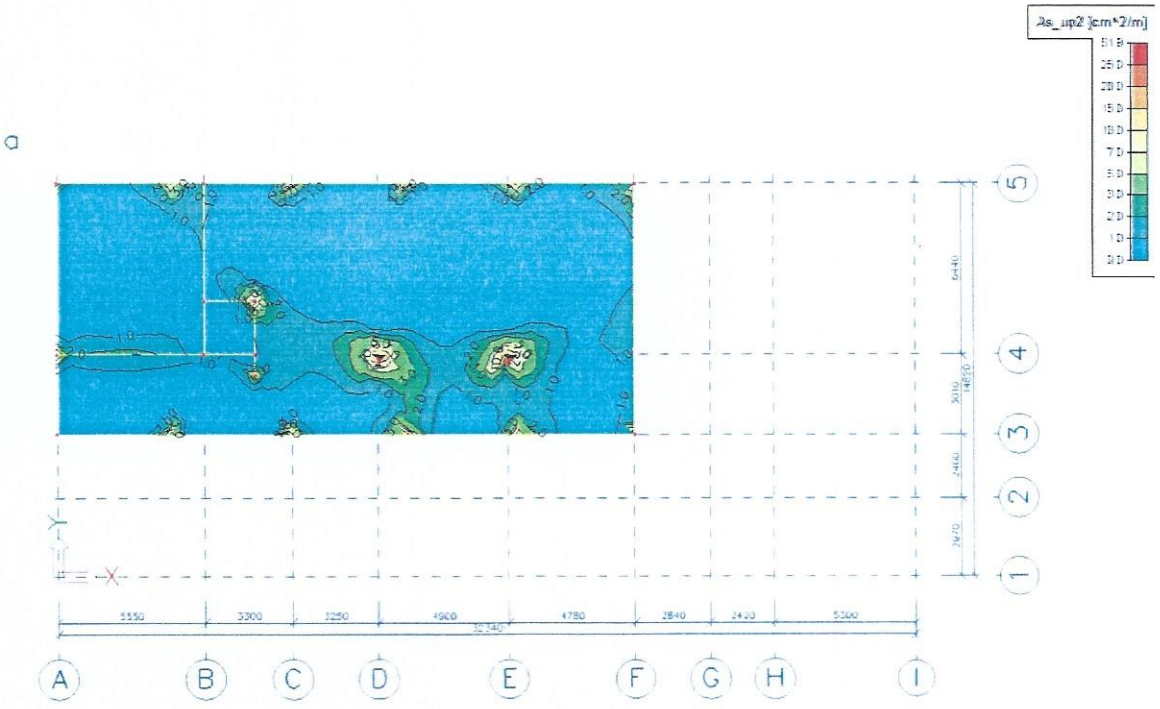
Schöck BOLE Version : 2.10.04

2.3.1. Nova plošča nad nadstropjem

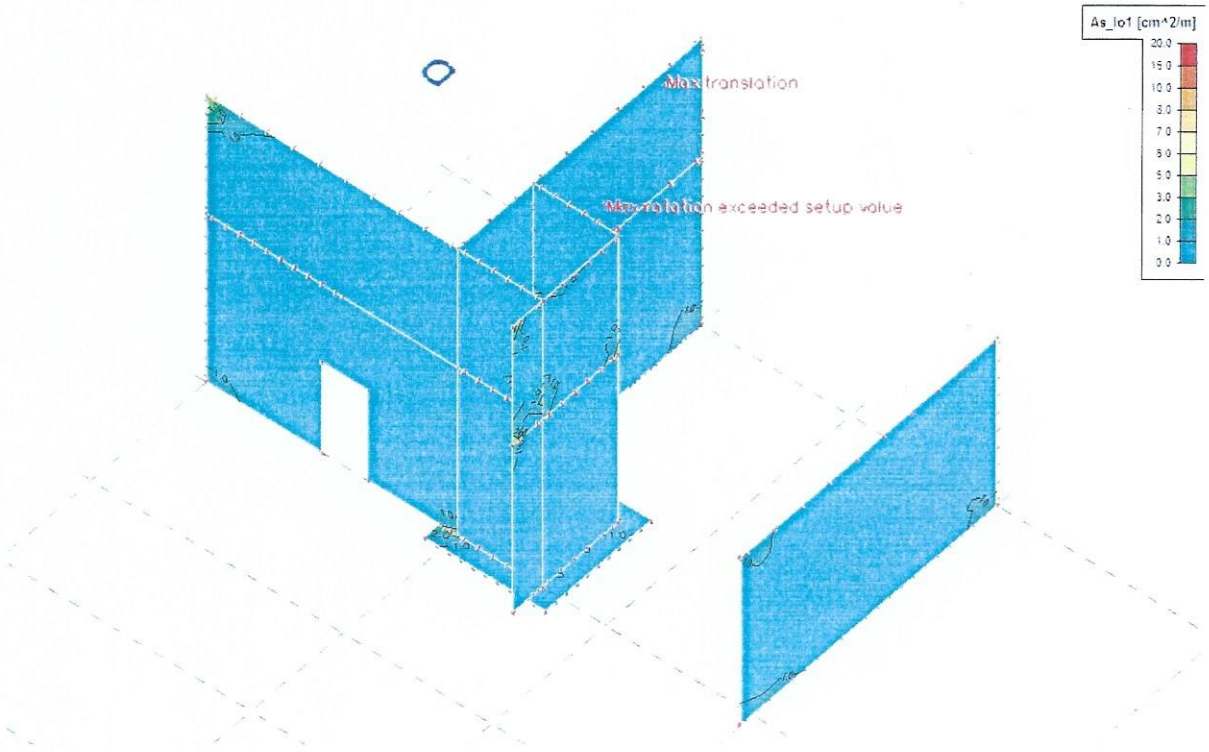


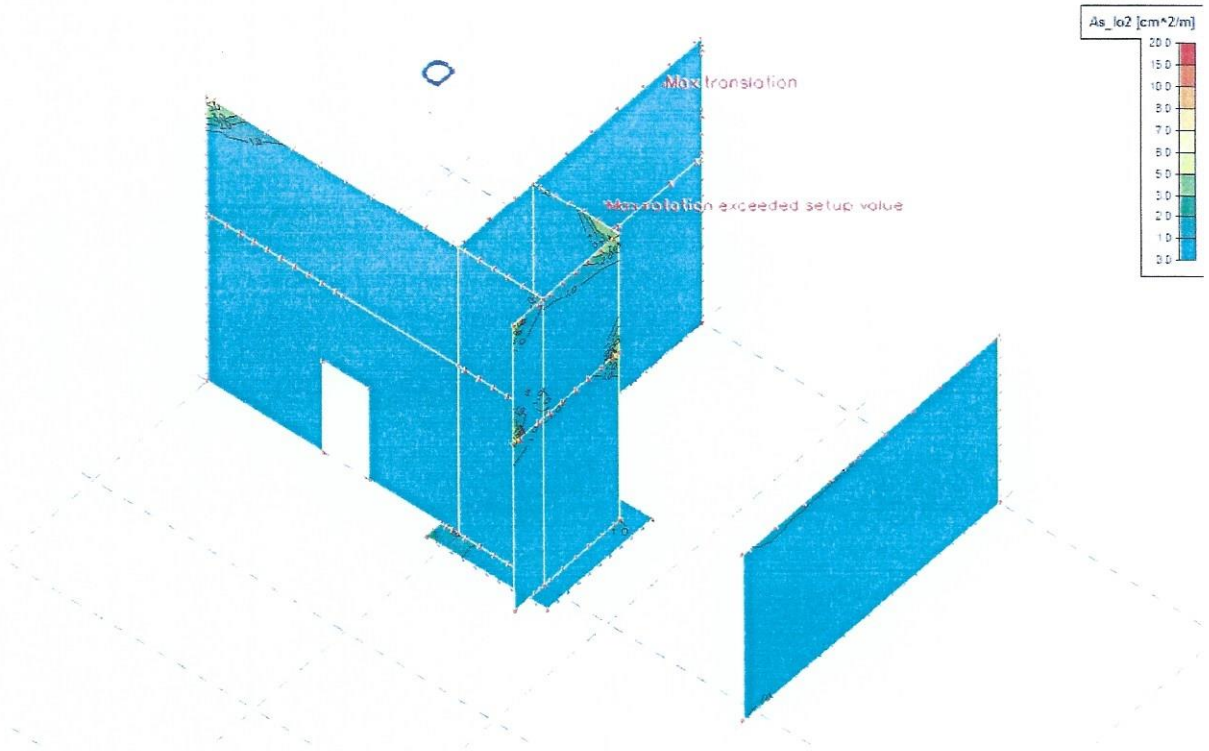


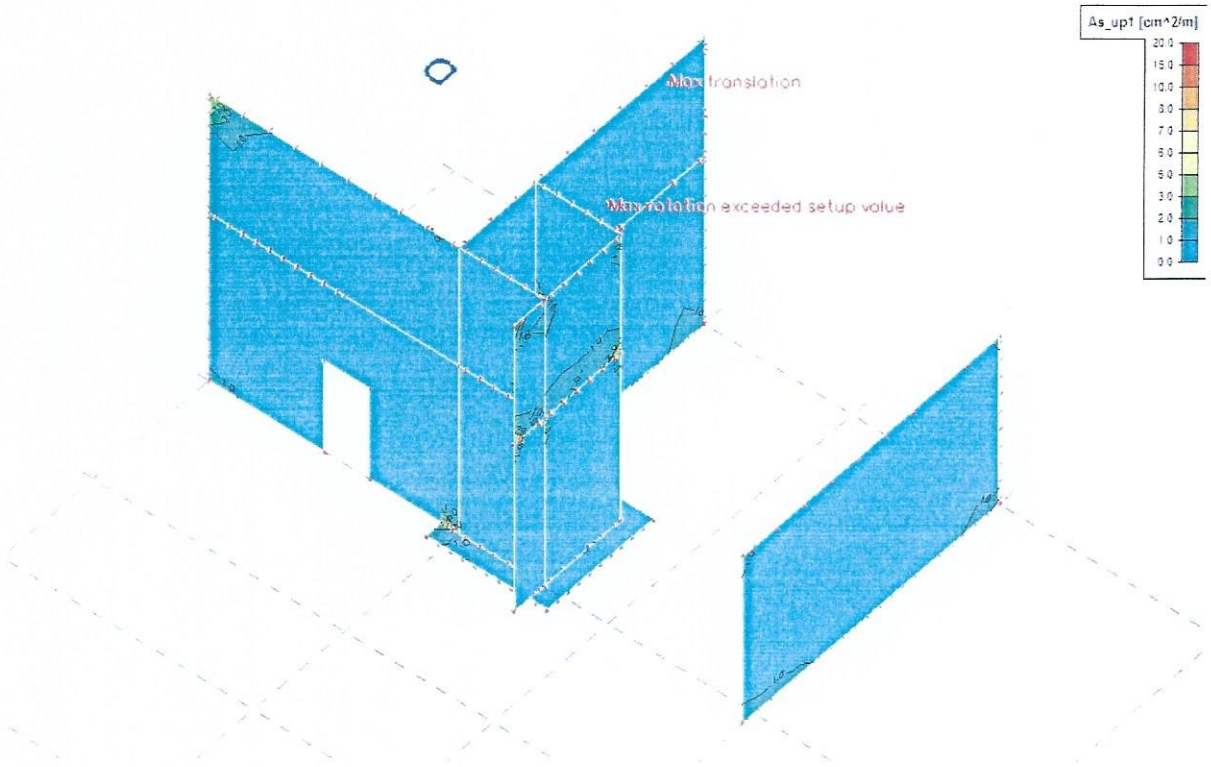

As_up1 [cm^2/m]
32.9
25.0
20.0
15.0
10.0
7.0
5.0
3.0
2.0
1.0
0.0
A
B
C
D
E
F
G
H
I
1
2
3
4
5


As_up2 [cm^2/m]
A
B
C
D
E
F
G
H
I
1
2
3
4
5

2.3.1. Vertikalni AB elementi



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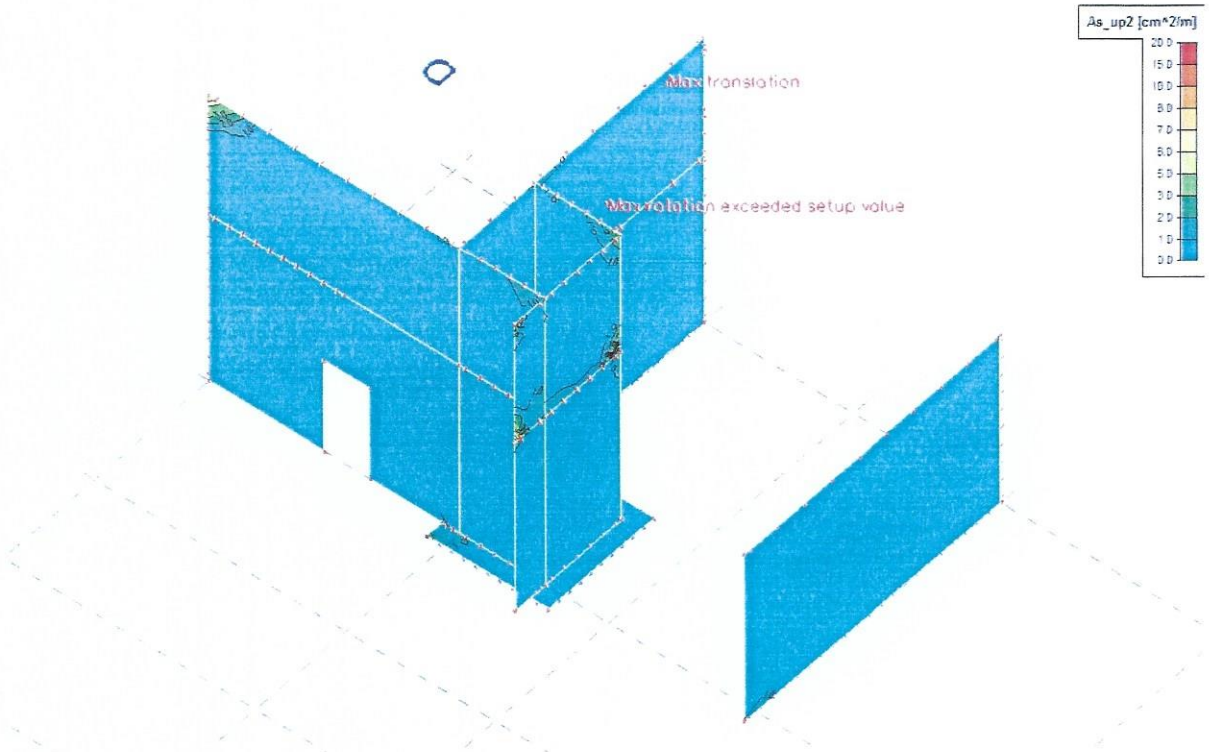
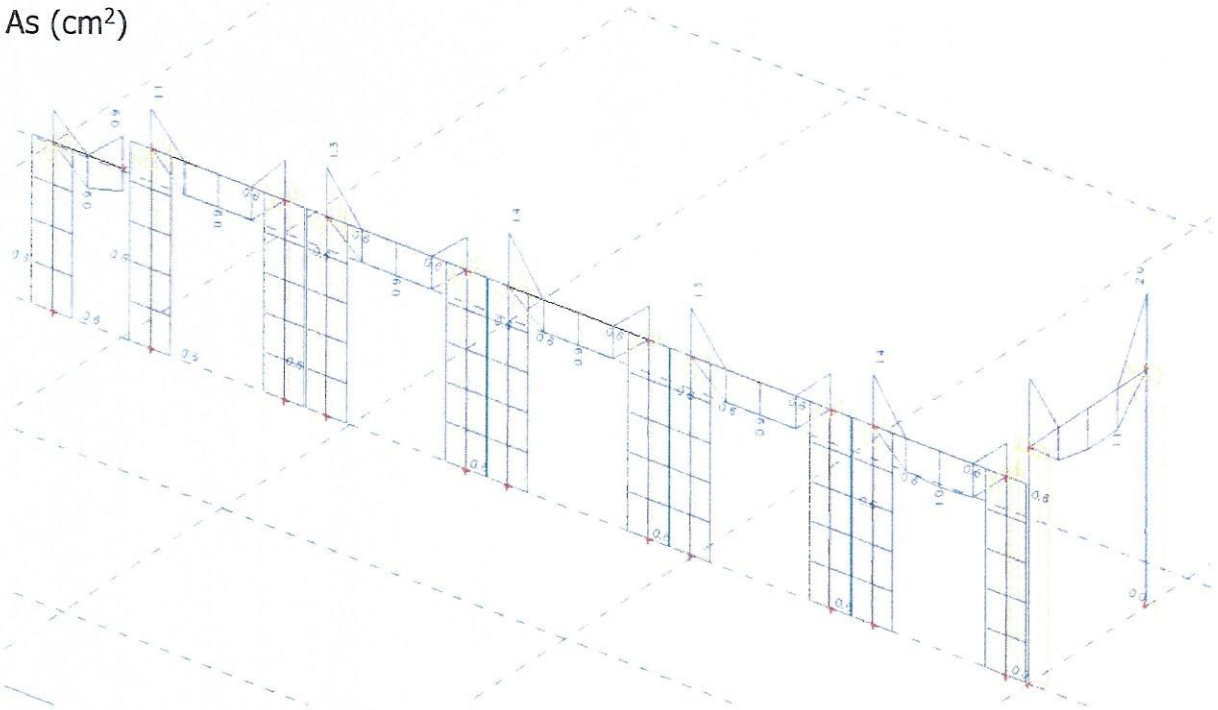
20.0
15.0
10.0
9.0
8.0
7.0
6.0
5.0
4.0
3.0
2.0
1.0
0.0
Max translation
Max rotation exceeded setup value

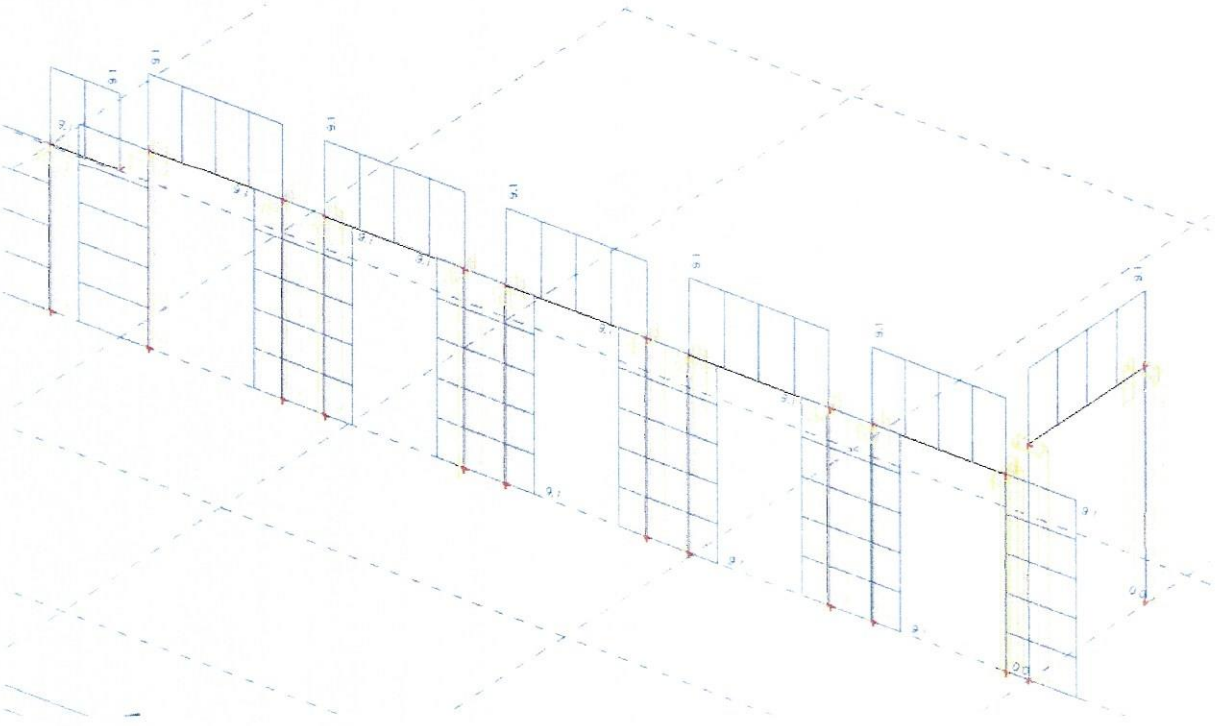

As_up2 [cm^2/m]
20.0
15.0
10.0
9.0
8.0
7.0
6.0
5.0
4.0
3.0
2.0
1.0
0.0
Max translation
Max rotation exceeded setup value

2.3.1. Okvirji okrog fasadnih prebojev

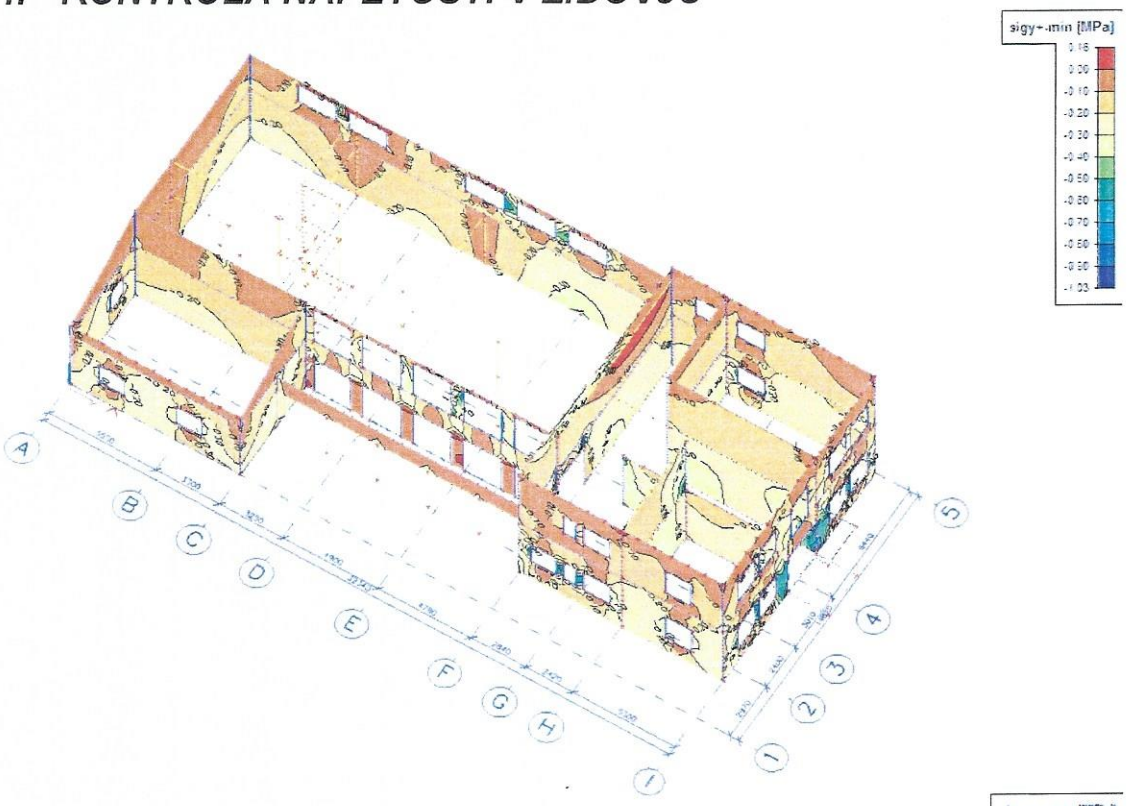
As (cm^2)

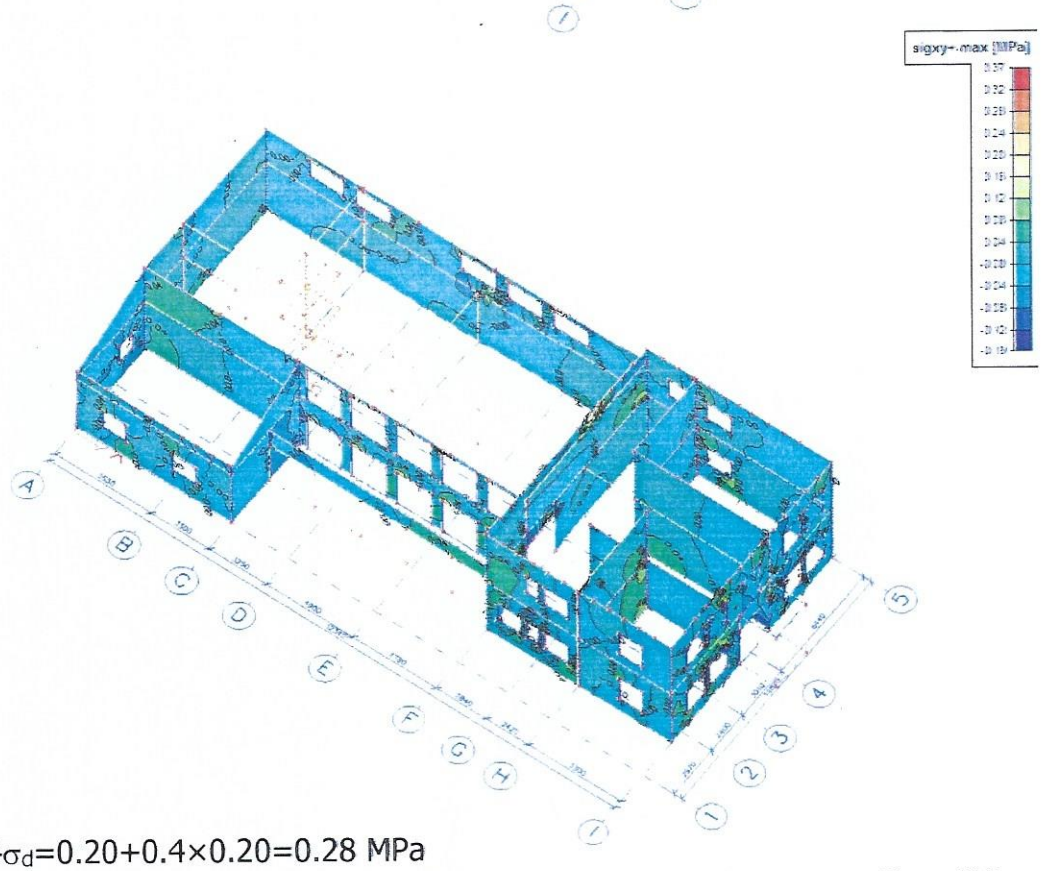


Ass (cm^2/m)



2.4. KONTROLA NAPETOSTI V ZIDOVJU





$f_{vk}=f_{vk0}+0.4\sigma_d=0.20+0.4\times0.20=0.28$ MPa

$f_{vd}=0.28$ MPa $/2.0 = 0.14$ MPa $> f_{v,Ed} = 0.10$MPa (brez lokalnih koncentracij) ... OK!